

Whitehead & Associates Environmental Consultants

Ashlee Rutherford Perception Planning P/L Re: Gresford Showground

Ref: 2971_WMR_Final

3rd November 2021

On-Site Wastewater Management Report (WMR) for proposed Site improvements at Gresford Showground, East Gresford, NSW

Whitehead & Associates Environmental Consultants Pty Ltd ("W&A") were engaged by Perception Planning P/L (the "Client") to prepare an On-Site Wastewater Management Report (WMR) for proposed site improvements at Gresford Showground; 29 Park Street, East Gresford, NSW (the "Site"). The Site, identified as Lot 7002 DP 96464 & Lot 1 DP11562, is approximately 11.35ha in area and is zoned RE1 (Public Recreation) under the Dungog Local Environmental Plan (LEP, 2014).

The Site is bound by private rural properties to the north & south, Allyn River to the east and Park Street to the west. Existing Site improvements include 15 buildings used for various purposes, including four (4) separate amenity blocks. Other improvements include two (2) lawn bowling greens, two (2) tennis courts, a skate park, animal yards and a rodeo arena. The remainder of the Site comprises general public space & parkland. A dam is located approximately 20m north of the Site on an adjacent property.

Comprising of two separate parcels, the Site operates as a regional agricultural show facility. The Site is owned by the NSW Government (Crown Land) and leased to the Gresford Park management trust (the "Trust") for public recreation purposes, including a range of community and (not-for-profit) commercial events. Potable water for the Site is sourced from roof (tank) water supply, with no reticulated sewer service available. The Site is identified as marginally flood-prone, per Council mapping, but no other major restrictions are noted.

W&A understand Perception Planning P/L is assisting the Trust to prepare a Development Application (DA) to Dungog Shire Council ("Council" or "DSC") for the construction of upgraded services and facilities at the Site including: an additional amenity block near the pavilion; a visitor carpark near the existing camp area; a disabled shower & toilet facility; formalisation of powered & unpowered campsites, and a caravan dump point. This report will also consider the future development of a proposed laundry, to be submitted under a separate DA. W&A understand the Trust has obtained external funding for wastewater servicing upgrades at the Site to facilitate these developments.

Council has adopted a comprehensive Development Assessment Framework (DAF) for onsite sewage management (OSSM), which sets out required standards for investigation, acceptable solutions and minimum standards for sewage management in unsewered areas of the Local Government Area (LGA). The DSC DAF (2015) identifies each allotment within the LGA as having Low, Medium, High or Very High hazard for on-site wastewater management. The Site is considered a 'High' hazard for non-domestic (commercial) OSSM.

DAF Minimum Standards for WMR (High Hazard – Non-Domestic)					
Report Element	Minimum Standard	Completed			
	 Name, contact details and qualifications of author(s). 	\checkmark			
	 Site location and owner. 	\checkmark			
Introduction and	 Allotment size (m² or ha). 	\checkmark			
Background	 Proposed / existing water supply. 	\checkmark			
	 Description of proposed facility (including equivalent persons). 	\checkmark			
	 Availability of sewer. 	\checkmark			
	 Broad overview of locality and landscape characteristics. 	\checkmark			
	 Details of the date and time of assessment in addition to statements confirming the methods used to complete the assessment. 	\checkmark			
	 Site assessment that considers all parameters listed in Table 6-1 of the DAF in accordance with AS/NZS 1547:2012. 	\checkmark			
Site and Soil	 Detailed review of available published soils information for the site. 	\checkmark			
Assessment	• Soil assessment that considers all parameters listed in Table 6-1 of the DAF in accordance with AS/NZS 1547:2012.	\checkmark			
	• Where multiple soil facets are present the site plan should show the approximate boundary between facets.	\checkmark			
	• Detailed explanation of the implications of observed site and soil features for system design and performance.	\checkmark			
	 Assessment of the existing condition of the receiving environment and sensitivity to on-site system impacts. 	\checkmark			
Quarterra	 Summarise potential treatment and land application systems considered including advantages and limitations. 	\checkmark			
System Selection	 Preliminary design calculations for a minimum of 2-4 options. 	\checkmark			
Clection	 Brief statement justifying selection of treatment and land application system. 	\checkmark			
	 Detailed wastewater characterisation (quality and quantity) including temporal variation using existing data for the subject site or similar facilities. 	\checkmark			
	 Establishment of clear, site-specific design criteria based on typical or published performance. 	\checkmark			
Design	• Process design in accordance with Tchobanoglous and Burton (2003) or Crites and Tchobanoglous (1997) detailing the rationale, assumed performance and capacity to manage design flows and loads. Process performance should be supported by published data or information that demonstrates the suitability of the process to the site and development.	\checkmark			
	 Daily water, nutrient and pathogen modelling to size any land application areas (see DSC Technical Manual). 	\checkmark			
	 Hydraulic design of collection, treatment and land application components to demonstrate viability of the process. Design drawings (CAD or similar) and specifications for all system components. 	\checkmark			
Site Plan	 Survey plan. Proposed allotment boundaries, dimensions and area; Location of existing buildings, swimming pools, paths, groundwater bores, dams and waterways; Location of exclusion zones (e.g. setback distances and unsuitable site and soil conditions); Location of all system components and any reserve areas to 	\checkmark			

The following table presents the minimum standards required by the DSC DAF (2015) for a 'high' hazard non-domestic development WMR.

	clearly demonstrate viability;	
	 Half metre elevation contours; and 	
	• Location of existing and proposed drainage pipework (centreline).	
Cumulative Impacts (Where required)	 Summary of approach taken and confirmation of compliance with the Minimum Standards documented in 3.2.4. Methodology documenting the basis and source of input data including reference to site specific data, published information or the Technical Manual to justify use. 	As per
	• Results demonstrating compliance with local water quality objectives and adequate management of health risk as defined and demonstrated in Table 2-15 and Section 10 of the Technical Manual.	discussion with Council
	• Brief discussion of long-term risks to health and environment and recommended management measures to address impacts.	
	Soil bore logs for all test pits.	\checkmark
Appendices	Raw laboratory results for soil analysis.	\checkmark
	• All design calculations and assumptions including screenshots of cumulative impact spreadsheets/models.	\checkmark

1 Author Statement

This WMR was prepared by Ben Colautti who is an Environmental Consultant with W&A, holding a Bachelor of Civil (Environmental) Engineering from the University of Technology Sydney. He has completed the On-site Wastewater Management professional short-course with the Centre for Environmental Training (CET) and has prepared WMR's for residential and commercial sites across the Central Coast and Hunter regions.

2 Introduction

This WMR has been undertaken in reference to the assessment and design principles of:

- AS/NZS 1547:2012 On-site Domestic Wastewater Management (Standards Australia/Standards New Zealand, 2012);
- Environment & Health Protection Guidelines: On-site Sewage Management for Single Households (Department of Local Government (DLG), 1998);
- Dungog Shire Council (2015) On-site Sewage Development Assessment Framework (DAF). Revision 3, dated 4 June 2015; and
- Dungog Shire Council (2015) On-site Sewage Management Technical Manual. Revision 2, dated 4 June 2015.

The following table presents information on the property investigated.
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Feature	Description		
Site Address	29 Park Street, East Gresford, NSW		
Lot / DP	Lot 7002 DP 96464 & Lot 1 DP11562		
Local Government Area	Dungog Shire Council		
Land Zoning	RE1 (Public Recreation)		
Lot Size (ha)	11.35		
Climate Zone category	Southern		

Sewer Connection Available	No
Potable Water Supply	Roof (tank) water supply

3 Site Description

Gresford Showground is a public recreation and parkland facility. Its predominant purpose is to provide the local residents of East Gresford, Gresford and the region with a place for community congregation, community events and general public recreation. A skate park, tennis courts, amenity blocks and camping grounds are available daily to the public. Other facilities on-site are specifically used for events and functions, these include storage buildings, pavilions, canteen and BBQ areas, club house, rodeo arena, grandstands and animal yards.

The community events are organised and/or permitted by the Gresford Park Trust, who source funding through donations, fundraising events and grants to organise events and functions for, and on behalf of, the community. Regular events held at the Site include motocross, equestrian, rodeos, mud runs and the local agricultural show.

There are four (4) amenity blocks on Site that service wastewater needs for visitors. Identified as Blocks 1, 2, 3 & 4, they are located as shown on Figure 1, Appendix A. Block 1 consists a single toilet within the tennis court clubhouse; Block 2 (known as the 'pavilion toilets') generally service event attendees, with some non-event visitor usage from campers and skaters; Block 3 (known as the 'pony club toilets') service event attendees, and Block 4 (known as the 'camping toilets') primarily service campground visitors (both event and non-event).

4 Site Usage

This section will outline W&A's determination of predicted visitor rates at the Site.

Aside from camping during the Easter and Christmas holiday breaks and periodic events throughout the year, the Site is assumed to have predominantly low usage.

Operational matters and assumptions were taken into consideration in the development of 'design' wastewater generation estimates for the Site. Where noted, the information is based on discussions with Site management (Gresford Park Trust). With uncertainties and limitations in data W&A have made some assumptions regarding visitor rates at the Site, W&A accepts that there will be some discrepancies in estimations and refers back to the client for confirmation.

4.1 Non-Event Attendance

Outside of formal 'event' periods, Site usage is typically comprised of the following use categories:

- Campers (short-term holiday or recreational);
- Skate Park users;
- Tennis court users and;
- Comfort stop (toilet break) or recreational users (dog walkers etc).

The Site experiences seasonal variation in attendance. A 'High' season period occurs from mid-December to mid-February (and the Easter break); 'Shoulder' season ranges from mid-February to March & October to mid-December, and the 'Low' season occurs outside of these times (April to September).

The following considerations and assumptions are made in regards to non-event usage, based on W&A analysis and anecdotal experience:

- Camp ground usage during non-event periods is seasonally dependant, with intraweek variability also noted:
 - For an average weekend day (Sat-Sun) during the Shoulder season ~30% of available camping sites are occupied, with the equivalent of ≈150 campers on-site.
 An average weekday (Mon-Fri) will see an occupancy of ~12%, with the equivalent of ≈60 campers on-site.
 - For an average weekend day during the Low season ~25% of the camping grounds are occupied, with the equivalent of ≈130 campers on-site. An average weekday will see an occupancy of ~2%, with the equivalent of ≈10 campers on-site.
 - For an average weekend day during the High season attendance increase to ~96% of the available camping sites, the equivalent of ≈500 people on-site. An average weekday will see an occupancy of ~67%, with the equivalent of ≈350 campers on-site.

Gresford Park Trust indicated that the Site experiences 300-500 visitors per day during Christmas break. With a typical breakdown of ~33% (167) of visitors utilising powered Sites and ~67% (333) of visitors utilising unpowered campsites.

- Skate Park users will be serviced by 'Block 2' facilities. Attendance is estimated as **10** (ten) visits per weekday and **30** for an average (shoulder) weekend day. With an assumed peak visitor rate of ≈50 people.
- There may be up to **15** weekday and **30** weekend day comfort stop and recreational users at the Site during an average (shoulder) period, these people will be entering the Site specifically to use the toilets or entering recreationally (e.g. walk the dog or exercising etc) and may make use of the facilities.

Seasonality is also expected to influence the number of Skate Park, comfort stop and recreational users. To allow for this variability, the visitation rates during a shoulder season (defined above) are increased by 30% to estimate high season attendance and reduced by 30% to estimate low season attendance.

 People using the tennis court will be serviced by 'Block 1' facilities. Tennis court usage is generally low with an approximate attendance of 8 (eight) weekday and 20 (twenty) weekend day visits. With an assumed peak visitor rate of ≈100 people for a busy day (i.e. tournament).

4.2 Event Attendance

Two (2) large annual events; the 'agricultural show' and the 'rodeo', as well as an array of smaller events and functions also occur at the Site. The Trust has indicated that both the Agricultural show and Rodeo attract approximately 5,000 people per event. Other small events, such as the Pony Club, Mud Run, Motocross and other (non-specific) community functions generally attract between 1,000-3,000 visitors per event. The majority of these events are 'annual' or part of a long-term calendar program for the Site.

The following considerations and assumptions are made in regards to events:

• If the event duration is ≥2 days, it is assumed that **80%** of event attendees will utilise available camping for at least 1 of the nights.

- If the event duration is <2 days, it is assumed that **20%** of event attendees will camp on Site.
- For event attendees who are day visitors (i.e. non-camping), it is assumed that ~75% will use the upgraded facilities at Block 2, with the remainder (25%) using Block 3 amenities.
- All events at the Site occur on a weekend, with 3-day events also including a Friday or Monday.
- Staffing is assumed for each event based on W&A estimations.

The Gresford Showground event schedule for 2021 (see Appendix E), along with attendance information provided by the Trust and the assumptions outlined, is used to inform Site usage during 'event' periods. These estimates are summarised in the following table.

Event	How long (days)	Patronage	Staff	Event Campers (Total)	Event patrons (Day Visit)
Pony Club (PRPC) (1 d)	1	300	5	60	240
Pony Club (PRPC) (2 d)	2	300	6	240	60
Dressage	1	500	6	100	400
RFS	1	500	6	100	400
Dungog Motorcycle	2	500	10	400	100
American Moto	2	1000	18	520	480
Mud Run	2	1000	18	520	480
Show (day 1)	2	2000	40	200	1800
Show (day 2)	2	5000	40	500	4500
Penning	2	500	8	400	100
Rodeo (day 1)	3	3000	40	200	2800
Rodeo (day 2)	3	5000	40	520	4480
Rodeo (day 3)	3	2000	40	50	1950
E' Zone	2	1000	10	520	480
PRPC Zone	2	300	6	240	60

4.3 Camping

Based on the (draft) Site Plan provided, camping facilities are to be predominantly located in the south-east portion of the Site. Proposed camping grounds and parking areas will occupy a large portion of undeveloped area in this location and will be responsible for a sufficient portion of sewage generated at the Site.

The following considerations and assumptions are made in regards to camping at the Site:

- Currently, ≈4,200m² of powered and ≈9,600m² of unpowered campsite area is available.
- The proposal will expand camping areas to 4,600m² (additional 400m²) of powered camping area and ≈14,000m² (additional 4,400m²) of unpowered camping area. Total area of available camping = 18,600m² (Perception Planning 'Draft Site Plan, Job No: J000480').
- A standard campsite is ~50sqm; however, due to informally organised camp grounds, pathways between Sites, surface rock, topography, vehicles/horse floats within the campsite and parked cars using campgrounds as parking. The footprint for each

campsite is taken as 90m². Therefore, the maximum number of available camping sites within the camping ground is estimated as 208 (18,600/90).

- The average number of visitors per campsite is assumed as 2.5, based on the fact that most campers will be couples with horse floats; therefore, the camp ground is assumed to have a maximum capacity of 520 (208 x 2.5) visitors. The assumed breakdown being 394 (unpowered) and 126 (powered).
- Caravans, RV users, people with fold out tents and horse floats are all included in the definition of "campers".
- Once proposed campgrounds have been fully developed, and assuming all (existing & proposed) campgrounds are occupied, the anticipated utilisation of facilities is as follows:
 - Block 2 will service ~32% of campers;
 - Block 3 will service ~8% of campers; and
 - Block 4 will service ~60% of campers.

5 Wastewater Generation

This section outlines the methods used to predict wastewater generation from existing and proposed uses at the Site, as well as how this wastewater is distributed between amenities.

5.1 Wastewater Quantity

5.1.1 Flow Allowances

Flow allowances for each development component were obtained from National and NSW guidelines, as referenced in the following table. Where necessary, estimates are provided based on W&A experience or anecdotal advice.

Development	Flow Allowance (L/Person/day)	Source
Powered Campsites	75	Septic Guidelines 2001 "laundry, showers and WC" (reduced from 86 to 75L from Water meter data, section 5.1.4)
Unpowered Campsites	27	Septic Guidelines 2001 "WC"
Event Attendees, Sport Facility & Recreational users	10	Table H4, AS/NZ 1547:2012 "meetings, roof tank supply"
Non-resident Staff	30	Table H4, AS/NZ 1547:2012 "non-resident staff"

Wastewater generation is calculated by multiplying the visitor rate (refer Section 4) for both non-event and event periods and the associated flow allowance for each contribution (refer table above)).

5.1.2 Non-Event Generation

From these values, seasonal non-event wastewater generation tables were developed for the Site. The tables approximate inter-week and seasonal variability at each wastewater generation point. For ease of analysis, generation for Block 1 is reported separately, while generation from Blocks 2, 3 & 4 is combined. Copies of each generation table produced are provided in Appendix B and summarised below.

		Lo	Low Should		Ilder		High	
		WW Gen (L/day)	working	WW Gen (L/day)	working	WW Gen (L/day)	working	
Skate Park	Weekday	70	(10 x 7)	100	(10 x 10)	130	(10 x 13)	
(L/day)	Weekend Day	210	(10 x 21)	300	(10 x 30)	390	(10 x 39)	
Recreational Users/Comfort	Weekday	105	(10 x 10.5)	150	(10 x 15)	195	(10 x 19.5)	
Stop (L/day)	Weekend Day	210	(10 x 21)	300	(10 x 30)	390	(10 x 39)	
Campers (L/day)	Weekday	3,900	(75 x 52)	4,680	(75 x 62.4)	15,402	(75 x 124)	
	Weekend Day	9,462	(75 x 124)	10,164	(75 x 124)	19,452	(75 x 124)	

5.1.3 Event Generation

Wastewater generation during individual 'events' is a function of both visitor and camping contributions, along with associated staff and (minor) non-event visitor contributions. For each event, as outlined in the Gresford Showground schedule, W&A made the following wastewater generation estimates:

Event	WW Gen (Camping - Powered)	WW Gen (Camping - Unpowered)	WW Gen (Camping - Total)	WW Gen (Day visitors)	WW Gen (Staff)	WW Gen (TOTAL)
Pony Club (PRPC) (1 d)	4,500	0	4,500	2,400	150	7,050
Pony Club (PRPC) (2 d)	9,300	3,132	12,432	600	180	13,220
Dressage	7,500	0	7,500	4,000	180	11,680
RFS	7,500	0	7,500	4,000	180	11,680
Dungog Motorcycle	9,300	7,452	16,752	1,000	300	18,060
American Moto	9,300	10,692	19,992	4,800	540	25,340
Mud Run	9,300	10,692	19,992	4,800	540	25,340
Show (day 1)	9,300	2,052	11,352	18,000	1,200	30,560
Show (day 2)	9,300	10,692	19,992	44,800	1,200	66,000
Penning	9,300	7,452	16,752	1,000	240	18,000
Rodeo (day 1)	9,300	2,052	11,352	28,000	1,200	40,560
Rodeo (day 2)	9,300	10,692	19,992	44,800	1,200	66,000
Rodeo (day 3)	3,750	0	3,750	19,500	1,200	24,450
E' Zone	9,300	10,692	19,992	4,800	300	25,100
PRPC Zone	9,300	3,132	12,432	600	180	13,220

5.1.4 Data Calibration

Available water usage data was used to 'quality check' W&A estimates for wastewater generation during both event and non-event periods. Water meter data was provided by the Trust based on two (2) registered meters; 'Arena' and 'Camp'.

A total of 5 useable months of data were obtained (mid-March 2021 to mid-August 2021) and were used to compare with W&A estimates for the corresponding periods (refer adjacent table).

Using this information, W&A found that generation was typically over-estimated during mid-level 'events' (i.e. Mud Run and Motocross) and flow allowances for camping contributions were adjusted accordingly.

Overall, good agreement was achieved between the observed water usage data and the W&A estimates during event periods.

Dates of	Time between	Total Flow	Average daily
readings	readings	for time	flow (L) within
(year-2021)	(days)	period (kL)	that period
18-Mar	7	116	16,580
29-Mar	11	128	11,640
8-Apr	10	121	12,100
19-Apr	11	191	17,370
27-Apr	8	85	10,630
10-May	13	58	4,470
13-May	3	11	3,670
16-May	3	30	10,000
25-May	9	73	8,120
28-May	3	30	10,000
31-May	3	54	18,000
8-Jun	8	77	9,630
21-Jun	13	117	9,000
30-Jun	9	40	4,450
13-Jul	13	66	5,080
19-Jul	6	17	2,840
26-Jul	7	60	8,580
3-Aug	8	44	5,500
10-Aug	7	47	6,720

5.2 Generation Distribution

Due to the dispersed nature of wastewater generation at the Site, particularly during 'event' periods when visitors are moving about, it was also necessary to analyse how wastewater generation would be distributed.

As previously described, with the exception of tennis court usage, all wastewater generated during both event and non-event periods would be portioned between Block 2, 3 and 4. To approximate the distribution of the total daily flow between each generation point, W&A used the following assumptions:

- Skate Park, recreational and comfort stop users will exclusively use the Block 2 amenities.
- Event attendees who are day visitors (non-campers), as well as event affiliated staff, are expected to predominantly (~75%) use the upgraded Block 2 amenities, with some overflow (25%) also using Block 3.
- Campers are expected to exclusively use the Block 4 amenities; however, Block's 2 and 3 are also expected to accommodate some overflow during periods of high campground utilisation.

The following table summarises the assumed proportion of camper usage between each amenity block depending on the range of total campers on Site.

	Block			
Campers on-site	2	3	4	
0 - 125	0%	6%	94%	
125 - 318	0%	3%	98%	
318 - 353	0%	13%	87%	
353 - 520	32%	8%	60%	

5.3 Summary

As shown, wastewater generation at the Site is highly variable, with long periods of low attendance punctuated by regular periods of short-duration moderate generation and occasional large events.

Based on estimates, annualised 'average' wastewater generation at the Site is expected to be ~**8,660L/day**, ranging from ~950L/day during the low-season and increasing to a maximum (peak) of up to ~**66,600L/day** during the Agricultural Show or Rodeo events. A graph summarising expected annual wastewater generation is provided in Appendix B.

The distribution of 'average' and 'peak' daily wastewater generation at each amenities Block (not including tennis court users), is summarised below.

	Block 2	Block 3	Block 4	Total
Average (L)	840	620	6,950	8,656
Peak (L)	35,100	12,700	18,800	66,592

5.4 Wastewater Quality

The contaminants in wastewater have the potential to create undesirable public health concerns and pollute waterways unless managed appropriately. As a result, domestic wastewater must be treated to remove the majority of pollutants and enable attenuation of the remaining pollutants through soil processes and plant uptake.

The majority of 'sanitary' wastewater generated at the Site is expected to be of 'typical' domestic nature, with combined wastewater streams; blackwater (toilet) and greywater (kitchen, laundry and shower) wastes. As such, untreated sanitary wastewater is expected to have characteristics similar to that described in the table below; which incorporates information taken from the NSW DLG (1998).

Parameter	Loading	Greywater %	Blackwater %
Daily Flow	8,660 L	65	35
Biochemical Oxygen Demand	200-300mg/L	35	65
Suspended Solids	200-300mg/L	40	60
Total Nitrogen	20-100mg/L	20-40	60-80
Total Phosphorus	10-25mg/L	50-70	30-50
Faecal Coliforms	10 ³ – 10 ¹⁰ cfu/100ml	Medium – High	High

5.5 Food Trucks

During events (especially the Rodeo and Agricultural show), a clubhouse with BBQ area and canteen will serve event attendees with cooked and uncooked food. Additionally, contract food service providers (food trucks) will be available on Site serving cooked food to attendees. Waste oil, grease and fat from Food Trucks must NOT be introduced to the OSSM system/s at the Site.

Management of these wastes remains the responsibility of the contractor provider and must be acknowledged in the license/approval conditions to attend each event. It is our understanding that most operators already adopt this approach and collect their waste oil and grease for off-site disposal.

5.6 Dump Point

A caravan dump point is proposed near the Block 4 Septic tanks. This will store caravan and RV collected wastewater to be removed via vacuum truck at a later date.

W&A have estimated the required size of the Dump Point tank based on the expected pumpout frequency; the average caravan/RV sullage tank size and the 'typical' frequency with which users will utilise the service.

The 'Dump point' pump out frequency is assumed to occur once (1x) per year, in coordination with other tanks prior to the event season. The average sullage tank size in the range of caravan/RV vehicles examined is ~80L. The frequency of (Caravan/RV) users making use of the Dump point facility has been estimated as ~240 separate uses per year.

This totals to a proposed dump volume of 19,200L (240 x 80L) per year. Based on this analysis W&A propose 20,000L of tank storage in the form of two 10,000L tanks to service as a Dump Point station.

6 Site and Soil Assessment

A Site investigation was undertaken by Connor Morton & Ben Colautti of W&A on the 28th July 2021. The following tables present the results of our site and soil investigation for the property.

A description of the Site physical constraints and the degree of limitation they pose to on-site sewage management (OSSM) is provided in the Table below. Reference is made to the rating scale in NSW DLG (1998) and, where appropriate, the DSC DAF (2015).

	SITE ASSES	SMENT		
Parameter	Data / Observation		Reference	Classification / Outcome
Climate	The Site experiences a temperate climate typical of south-eastern Australia. Median annual rainfall for the Site is 882.2mm. Monthly rainfall ranges from 31mm in August to 86.8mm in March. Mean annual pan evaporation for the Site is 1568.5mm. Potential evaporation exceeds rainfall for all months of a typical year at the Site.		BOM Stations: 61024 (Rainfall) and 061288 (Evaporation)	Minor limitation
Hydraulic balance (daily) attached: Yes				
Nutrient balance (annual) attached:		Yes	per DSC DAF (2	2015) procedure
Land applicatior	n area (LAA) sizing attached:	Yes		
Wet weather sto	prage requirement:	No	N/A	
Flooding			W&A analysis	
Land application	n area above 1:20 ARI flood level:	Yes	from previous job along the	Minor
Land application	n area above 1:100 ARI flood level:	Likely	Allyn River (Ref: Job No. 1471)	limitation
Electrical compo	onents above 1:100 ARI flood level:	Yes	1471)	
Exposure	The Site is predominantly cleared of Good exposure to sun and prevailing	Ū	Minor limitation	
Slope	Ranges from 1% to 8% within the 'a effluent management areas (EMA).	vailable'	Minor to Modera	te limitation
Landform	Undulating hills generally <10% slop	pe,	Minor limitation	
Run-on and Seepage			Minor limitation	
Erosion Potential	None observed in available EMA.		Moderate limitat	ion
Site Drainage	Moderately well drained. No signs o saturation; however, some mottling		Minor to Modera	te limitation

	SITE	EASSESSM	ENT		
Parameter	Data / Observation		Reference	Classification / Outcome	
	observed in subsoil horizons within the proposed LAA, indicating imperfect drainage at times during the climate cycle.				
Fill	None observed or apparen	t.		Minor limitation	
Groundwater	No shallow groundwater encountered during soil survey to ~850mm in proposed LAA (BH1). NSW Office of Water groundwater bore registry indicates no bores are located within 250m of the Site. The NSW DLG (1998) recommended 250m buffer distance to domestic groundwater bores can therefore be achieved within the available EMA.		Minor limitation		
Buffers Applica					
Permanent river	s and creeks (100m):	Yes	Achievat	ble (shown on Site	Plan).
Intermittent creeks and drainages (40m):		Yes	Achievat	Achievable (shown on Site Plan).	
Domestic groun (250m):	dwater wells and bores	N/A			
Other sensitive	receptors:	N/A			
Lot boundaries (if EMA upslope)	(3m if EMA downslope-6m :	Yes	Achievable (shown on Site Plan).		Plan).
	ways and swimming pools nslope-6m if EMA	Yes	Achievable (shown on Site Plan).		Plan).
Limiting horizon etc.) (0.6m):	(groundwater, bedrock	Yes	Achievat	ble with preferred l	LAA type (SSI).
Surface Rock / Outcrop Surface rock (300-350mm deep) and rock outcrops were observed throughout the west portion of the Site. These areas have been excluded from 'usea land' calculations.		western	Major limitation		
Available EMA			Moderate to Maj	or limitation	
Concluding Re	marks				
Overfaces and to in				d limitationa con	

Surface rock is a major constraint to OSSM at the Site. Identified limitations can be successfully avoided and/or mitigated by OSSM design.

	SITE ASSESSMENT		
Parameter	Data / Observation	Reference	Classification / Outcome
Available EMA OSSM design.	is identified in the eastern portion of the Site, sub	ject to on-site flow	w balancing and

	SOIL ASSESSMENT (physical)		
Parameter	Data / Observation	Reference	Classification / Outcome
Soil Depth	350mm (BH3) due to river cobble, stiff bedrock & weathered bedrock respectively. BH1		on
Soil Profile	 BH1 A₁: 0 - 350mm, weakly structured light clay (Cat 5) B₁: 350 - 850mm, massive medium clay (Cat 6) BH2 A: 0 - 300mm, massive sandy clay (Cat 5) BH3 A: 0 - 350mm, weakly structured sandy clay loam (Cat 4) Soil borelogs presented in Appendix C 	Major limitation Mitigation available through design.	
Depth to Water Table	No shallow (episodic) water table encountered in any BH.	Minor limitation	
Coarse Fragments (%)	0-50% (<200mm), cobblestone in EMA.	Moderate limitat	on
Soil Permeability	< 0.06m/day (inferred)	Based on massive medium clay (Cat 6) subsoil	Major limitation
Modified Emerson Aggregate Class (EAT)	Topsoil: 3(2)-2(1) (slight to moderate dispersion) Subsoil: 5 (stable)	Moderate limitation Mitigation recommended (see Section 10.1).	
Soil Landscape	The Site is located within the <u>Gresford (gd)</u> and <u>Paterson River (pa)</u> Soil Landscapes. The 'gd' soil landscape is located on rolling low hills to hills on Carboniferous sediments. Slopes are typically <25%, with relief 40-160m and elevation 80-200m. Groundcover consists of cleared tall open forest and rock outcrops occur occasionally on crests. Soils are typically moderately deep and moderately well-drained Natric Brown Kurosols, with shallow, moderately	Dungog 1:10 (Henders	0,000 Sheet on, 2000)

well-drained bleached Leptic Tenosols on upper slopes and crests.	
The 'pa' soil landscape located on the narrow to moderately broad floodplains on Quaternary alluvium in the Gresford Hills and Williams Range regions along the Paterson and Allyn Rivers. Deep, rapidly drained Stratic Rudosols (sandy Alluvial Soils) on levees and recent alluvial deposits along channel banks. Deep, well-drained Brown Dermosols (Brown Earths) on alluvial plains.	

Concluding Remarks

Site soils are predominantly characterised by the 'Gresford' soil landscape; sandy clay loam to sandy clay topsoils (Cat 4/5) to ~100-350mm depth, underlain by weather (sandstone) parent material. The soil Landscape transitions to 'Paterson River' along the Allyn River; these soils involve weakly structured light clay topsoil underlain by massive medium clay subsoil (~300-700mm depth). Soil structure is typically weak to massive. The Available EMA sits on the boundary between the two soil landscapes but is reflective of 'Paterson River'.

Based on identified soil characteristics a (maximum) design irrigation rate (DIR) of 2mm/day is recommended for irrigation systems and a (maximum) design loading rate (BLR) of 5mm/day is recommended for subsoil absorption systems, with reference to Tables M1 and L1 AS/NZS 1547:2012 for the limiting Cat 6 subsoil.

Soil conditions are generally moderate in the available EMA; instability, coarse fragments (cobblestone) and permeability limitations present.

Potential negative consequences associated with coarse fragments, soil dispersion (EAT) and instability can be mitigated through appropriate LAA placement and soil improvement measures (see Section 10.1). Identified soil permeability limitations will be mitigated through conservative LAA sizing and design.

	SOIL A	SSESSMENT (chemical))	
Parameter	Data / Observation		Reference	Classification / Outcome
рН	Topsoil: 6.73-6.85 Subsoil: 6.78	Neutral	Minor limitation	
EC (EC _e)	Topsoil: 0.6-0.784 Subsoil: 0.329	Non-saline	Minor limitation	
ESP (%)	1.7	Non-sodic	Based on soil laboratory results for	Minor limitation
CEC (me/100g)	9.1	Very Low Fertility	samples taken from a nearby site on the same	Major limitation
P-sorption (mg/kg)	111	Low	soil landscape (1471_East Gresford_2015)	Major limitation

Concluding Remarks

Very Low fertility and low p-sorption capacity of the soils pose major limitations to OSSM within the available EMA. Practices to mitigate these limitations are outlined in section 10.1.

7 Existing OSSM Arrangements

This section describes the existing OSSM systems as well as performance of treatment units and LAA's on-site.

At present, three (3) separate (stand-alone) OSSM systems are operating at the Site, each receiving wastewater generated from one or more amenity blocks. OSSM 1 services the tennis court toilet (Block 1), OSSM 2 services the Pavilion amenities (Block 2) and OSSM 3 services both the Pony Club and Camping amenities (Blocks 3 and 4, respectively). The arrangement of current OSSM systems at the Site is provided as Figure 2, Appendix A.

7.1 OSSM 1

Wastewater from Block 1 is treated in a \approx 2,000L concrete septic tank (Ø1.4m), followed by a \approx 1,500L collection well, before being displaced to a \approx 150m² (10m x 15m) absorption bed located south-east of the tennis courts. W&A estimate the peak hydraulic load to this OSSM system is ~1,000L/day (\approx 100 visitors during tournament).

The existing septic tank is considered to be appropriately sized for the maximum hydraulic load expected, providing in excess of 2 days hydraulic residence time (HRT) for effective primary treatment. Both the septic tank and collection well were in good condition and the subsoil LAA displayed no signs of seepage or pooling.

W&A consider OSSM 1 suitable for continued use at the Site, providing that the treatment tanks and LAA are suitably defended from damage or disturbance. W&A recommend the existing LAA is fenced off from access to prevent accidental damage from parking.

7.2 OSSM 2

Facilities in the 'Pavilion' amenities block (Block 2) include 8 toilets, ~3m of urinal and 3 handbasins. Available facilities will be expanded as part of the current project to include an additional 7 toilets (including a disabled toilet) and 5 handbasins. Wastewater generated from Block 2 is currently treated in a ~2,100L (Ø1.65m) concrete septic tank, followed by a ~2,100L (Ø1.65m) collection well. Treated effluent is then displaced into a ~155m² (12m x 13m) absorption bed located approximately 20m east of the block.

Both existing tanks are old, displaying signs of concrete cancer and are considered to be significantly undersized for the expected hydraulic loads. The existing LAA is heavily overgrown with vegetation and shows significant signs of overloading with seepage and pooling along the northern boundary. Investigation of historical aerial photography suggests occasional effluent seepage away from the LAA into a drainage channel on an adjacent property and towards the Allyn River. The LAA is located within the 40m buffer zone from the drainage channel and 6m buffer from the property boundary recommended by the NSW guideline (DLG, 1998).

W&A consider OSSM 2 no longer serviceable and unable to meet the future needs of the Site. The LAA is not compliant with current regulatory guidelines or Council policy (DSC DAF, 2015) and should therefore be replaced.

7.3 OSSM 3

Facilities in the 'Pony Club' amenities block (Block 3) include 6 toilets and 2 handbasins. No changes to this amenities building are proposed under the current project. Wastewater generated from Block 3 is currently treated in a \approx 2,700L (Ø1.65m) concrete septic tank,

followed by a ≈2,700L (Ø1.65m) pump well. A submersible (Mono) pump transfers primary effluent from the pump well to a combined LAA south of Block 3, via 32mm PVC pipe.

Facilities in the 'Camping' amenities block (Block 4) include 6 toilets, 4 showers and 3 handbasins. No changes to this amenities building are proposed under the current project. Wastewater generated from Block 4 is currently treated in a \approx 7,500L (Ø2.25m) concrete septic tank, followed by a \approx 2,700L (Ø1.55m) septic tank and \approx 2,700L (Ø1.55m) pump well. A submersible (Mono) pump transfers primary effluent from the pump well to a combined LAA south of Block 3, via 32mm PVC pipe.

The existing (combined) LAA for OSSM 3 is approximately 500m² in area (22m x 23m) and comprises a subsoil absorption bed. The LAA is located ~40m west Block 4 and forms part of the preferred location for effluent application at the Site. The existing LAA is heavily overgrown with vegetation and shows significant signs of overloading with pooling in the north-east. Settling and soil loss are apparent, with gravel distribution aggregate being close to or exposed at the surface in areas. The LAA is located outside of the 100m buffer zone from the Allyn River as recommended by the NSW guideline (DLG, 1998).

7.4 Summary

While several of the concrete (septic/collection well) tanks are in working order, analysis suggests they are significantly undersized for the expected hydraulic loads and should be replaced. Similarly, the existing LAA appears under-sized and overloaded. Replacement and upgrade are recommended.

8 Proposed OSSM System

This section describes proposed changes to OSSM at the Site.

Given that existing LAAs for OSSM 2 and OSSM 3 are performing poorly and a number of the existing concrete tanks are no longer serviceable or significantly undersized, it is recommended that the system is re-designed to incorporate an appropriate level of treatment, followed by sufficient storage to prevent hydraulic overloading of the effluent LAA during periods of heavy use.

Where possible, the proposed upgrades will utilise or re-purpose existing infrastructure.

8.1 Design Overview

The primary objective of the proposed OSSM upgrade is to simplify servicing arrangements by providing adequate pre-treatment and transfer of effluent to a centralised treatment system capable of reliably producing 'primary' effluent quality suitable for long-term effluent land application at sustainable loading rates. The proposed OSSM system aims to: (i) minimise the impact to Site users; (ii) simplify maintenance and management, and (iii) comply with Council's current regulatory requirements regarding environmental and public health protection. To achieve these objectives, the recommended upgrade design will be as follows:

- Block 1 will continue to be serviced by the existing stand-alone OSSM system (OSSM 1) to manage generated wastewater from the tennis court users;
- (ii) OSSM 2 and OSSM 3 will be modified and consolidated to one (1) central OSSM system servicing Blocks 2, 3 & 4; and
- (iii) generated wastewater from Blocks 2, 3, and 4 will be stored in balance tanks (as required), with controlled application to a new consolidated LAA.

8.2 Proposed OSSM Upgrades

Proposed upgrades to OSSM at the Site will include the following wastewater servicing infrastructure (refer to Figure 3, Appendix A):

8.2.1 Pre-treatment

To prevent issues with debris (rags, floatable materials etc.) causing problems with the sewerage reticulation and transfer system, ('interceptor') septic tanks will be used at Blocks 2, 3 & 4 (see Figure 3, Appendix A for reference). As appropriate, the existing arrangements may be retained; however, the following additions are recommended:

- All existing tanks at Block 2 will be removed and replaced with one (1) 10,000L septic tank and one (1) 10,000L collection (pump) well; and
- The existing septic tank at Block 3 will be removed and replaced with one (1) 10,000L septic tank. The existing 2,700L collection (pump) well will be retained.

8.2.2 Collection and Transfer

Pre-treated wastewater generated from each amenities block will be collected in an adjacent collection (pump) well for pressure transfer to a central treatment facility (STP).

8.2.2.1 Pumps

Each collection (pump) well will be fitted with dual (duty/standby) macerating pumps with the capability to pump fine solids to the STP.

The proposed arrangement will minimise the risk of blockages and the redundancy configuration shall be automated to ensure load switching in the event of failure of each individual pump set.

The pumps will be demand (float switch) controlled, with high-level override to utilise both pumps in the event of 'peak' flow conditions. This will ensure that during high flow periods the collection (pump) wells will not exceed capacity or contingency if one pump fails.

8.2.2.2 Sewerage

The majority of the sewerage network (pipework) is proposed for replacement under the proposed OSSM upgrade for the Site. New infrastructure will include:

- Suitably-sized pressure rising mains to transfer pre-treated effluent from the collection (pump) wells at Blocks 2, 3 and 4 to the inlet of the STP;
- Replacement or realignment of the drain from Block 2 to the new 10,000L septic tank at Blocks 2 and 3; and
- Installation of a new service line to distribute effluent to the (upgraded) effluent LAA

8.2.3 STP Design

As detailed (Section 5.3), 'average' wastewater generation at the Site is expected to be \sim **8,660L/day**, ranging from \sim 950L/day during the low-season and increasing to a maximum 'peak' of up to **66,600L/day** during the Agricultural Show or Rodeo events. Based on traditional sewer design procedure, instantaneous flow values may be \sim 6,000L/hour for short periods.

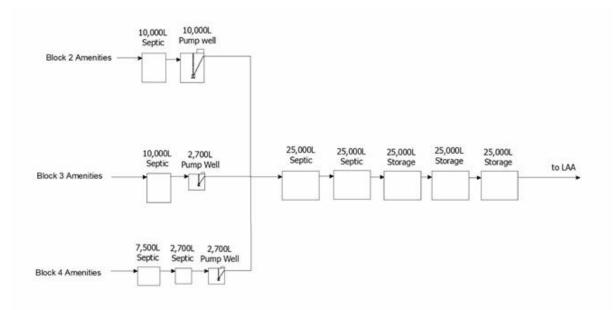
Primary treatment is aimed at the removal of dissolved and suspended organic material by a combination of physical and biological methods, including settling of solids and anaerobic microbial digestion. For effective primary treatment, it is generally accepted that the hydraulic retention time (HRT) within a treatment system should be greater than 24 hours, although this

may be reduced for short periods without significant long-term impact on treatment performance.

The recommended STP design comprises the installation of additional 'primary' septic tanks in a centralised location to service (combined) wastewater generation from Blocks 2, 3 and 4.

W&A recommend that the size of the STP is based on achieving a (minimum) 'primary' treatment effluent standard to minimise the risk of long-term impact to the new LAA. Therefore, to achieve this outcome, a minimum of two (2) 25,000L septic tanks are proposed. The tanks shall be installed in-series, effectively creating a (2x) baffled 50kL primary treatment STP. The preferred location for the installation is east of the animal yards.

The exact positioning of the new tanks will depend on the local gradient and level controls and can be determined in consultation with a licensed plumber and Council prior to obtaining consent for the installation. See Figure 3, Appendix A for proposed tank locations.



The below schematic shows the proposed OSSM network design:

8.3 Treated Effluent Quality

With proposed upgrades, the existing STP is expected to reliably produce effluent of (minimum) primary standard, suitable for further treatment and dispersal within the preferred effluent land application system (absorption beds).

Parameter	Expected Effluent Quality
Biochemical Oxygen Demand	~150mg/L
Suspended Solids	~50mg/L
Faecal Coliforms	>10,000cfu/100mL
Total Phosphorus	≤15mg/L
Total Nitrogen	≤60mg/L

The listed phosphorus and nitrogen concentration values are upper limits (only) and have been adopted for nutrient balance modelling.

8.4 System Operation and Management

Successful performance of wastewater treatment systems relies on periodic monitoring and maintenance, which will be the responsibility of the Owner. The treatment systems should be serviced by a suitably qualified technician at the prescribed intervals.

8.4.1 Pump-out

W&A propose a 'strategic' pump-out should occur once a year just prior to the Rodeo. This is proposed as 2 (two) 20,000L¹ tanker pump-outs. One of which will empty or significantly reduce the volume within the interceptor (septic) tanks immediately outside of amenities Block 2, 3 & 4. The other 20,000L tanker will empty the caravan dump point tanks.

The 'interceptor tanks' outside each amenities block will experience wastewater flows throughout the year which will periodically exceed the recommended 24-hour hydraulic residence time (HRT) this is especially predicted to occur at Block 4. The sludge build up will be high in these tanks and will require more desludging than normal. By desludging before the Rodeo once a year, it is both removing solid build up within the tanks (which increases the tanks effective volume) and creates air space to be replaced with wastewater during an event day.

Adopting a 'strategic' pump-out approach will also remove large debris (rags, cloth etc.) from the waste stream and assist in preventing blockages of the respective pump stations for each Block.

8.5 Contingency Plan

W&A have designed the OSSM system to be able to manage a peak Holiday day as well as a peak 'Event' (i.e. 3-day Rodeo) with one pump-out over the year (prior to the Rodeo).

However, it should be noted that the anticipated wastewater loads are based on expected visitor numbers as outlined in Section 4. In particular, the assumed (maximum) attendance value of 5,000 visitors for large events (Rodeo and Agricultural) is critically important. If these values are likely to be exceeded, W&A recommend the development of a Contingency Plan for managing excess wastewater generation on these days.

W&A recommend the Trust seek a 'guarantee of service' contract with a local pumpout/vacuum truck contractor. This will ensure that sewage can be removed conveniently from the OSSM system during the Rodeo or any other future multiple day event.

9 Proposed Effluent Management

This section describes the Sites capability for effluent management and provides design details, including sizing of the required LAA. As detailed above, Primary treatment has been considered for the Site.

¹ A 20,000L pump out is roughly equivalent to the daily wastewater produced from 1,000 event day visitors.

9.1 Onsite Effluent Management Options

W&A have considered the suitability of various land application systems in relation to the identified Site and soil limitations. In determining the suitability of the various options, W&A have assessed the Site constraints and the relative environmental and public health risks associated with each.

The table below provides a summary analysis of the range of effluent land application options considered, and presents recommendation for the preferred approach to be used in conjunction with primary treatment systems on-site.

Land Application Option	Suitable	Reasoning
Absorption Trenches/Beds	Yes (With detailed design and mitigation)	Site soils within the EMA are not conducive with absorption systems. However, a suitably textured imported fill raised at a suitable height above the limiting clay layer will allow for Absorption Bed Construction within the EMA. Along with an appropriately conservative Design Loading Rate (DLR) and effluent storage allowance.
ETA Beds	Possible	While allowable, would have relatively higher construction costs and lower effluent load rate.
Mounds	Possible	Considered suitable, but discounted due to their substantial cost.
Surface Irrigation	No	Not enough EMA
Subsurface Irrigation	No	Not enough EMA

Due to limited available EMA at the Site and observed Cat 6 subsoils, a raised pressure-dosed absorption bed is the preferred effluent land application option for the Site. A description of the preferred effluent management method and (nominal) sizing are presented below.

9.2 Buffers

Buffer distances from LAAs are recommended to minimise risk to public health, maintain public amenity and protect sensitive environments. Buffer or setback distances are recommended to provide a form of mitigation against unidentified hazards and reduce potential pathways of human and environmental exposure.

The following environmental buffers are required, based on Table 6-8 of the DSC DAF (2015):

- 250m from domestic groundwater bores;
- 100m from permanent watercourses;
- 40m from intermittent watercourses and dams;
- 6m if area up-gradient and 3m if area down-gradient of driveways, swimming pools and buildings;
- 12m if area up-gradient and 6m if area down-gradient of property boundary; and
- 0.6m vertical separation from hardpan or bedrock.

All recommended buffer distances are achievable on-site, besides that to intermittent watercourses (drainage lines), as shown on the Site Plan (Appendix A, Figure 1).

9.3 Land Application Area (LAA) Sizing

Daily soil-water and nutrient balance modelling were undertaken to determine the sustainable application rate for Site soils and to estimate the necessary size of the upgraded LAA to manage the 'design' hydraulic and nutrient loads from the Site

9.4 Daily Modelling Overview

The Land Application Mass balance (LAM) is a Microsoft Excel based daily water, nutrient and pathogen mass balance model developed by BMT WBM for predicting the performance of OSSM systems under varying environmental conditions. The algorithms in the model have been derived from the Decentralised Sewer Model (DSM) and tailored to suit a single site application. It can assess long-term environmental and human health performance of wastewater systems.

The LAM requires a range of bio-physical parameters as inputs to determine whether a LAA option would be sustainable at the Site. The model predicts OSSM performance by simulating the movement of pollutants within the effluent load as it travels from the point source (on-site or community-scale systems) as surface or subsurface flows. The LAM does not predict the minimum area required to achieve zero surface runoff or deep drainage, instead, like the nominated area approach of the monthly water balance, the model predicts the surface and subsurface discharges based on a set of nominated conditions such as receptor sensitivity, soil, slope, climate, wastewater input and available area.

A summary of the model processes, inputs and results is provided below.

9.4.1 Model Inputs

The simulations were run for a data period of 60 years (1961-2021) and represent a conservative estimate of long-term performance based on available information and a set of assumptions as detailed.

Simulations were carried out for the preferred land application, as follows:

• Run 001 modelled flow into the LAA

Daily climate data used in the model was sourced from 'SILO Data Drill' information available through the QLD Department of Environment and Science. The adopted SILO data set uses the (FAO56) Penman-Monteith methodology to estimate reference evapotranspiration (ET_0), which is a function of both evaporation and transpiration factors, based on a specific reference crop planted in the LAA (assumes turf).

Rather than simplistic loading rates, as utilised in monthly modelling, the LAM inputs include a more detailed estimation of the soils ability to receive, store and transmit water by approximating parameters such as effective saturation, field capacity, and the infiltration exponent. Soil input data is based on Hazelton and Murphy (2007) soil data guide and soil investigations undertaken within the EMA for the Site. The proposed fill and the underlying limiting natural topsoil was used to define soil input data (moderately structured loam & light clay respectively).

Phosphorus sorption (P-sorption) data was obtained via 5-point isotherm analytical results taken from a composite soil sample collected nearby (by W&A) and analysed by an independent NATA accredited laboratory (Lanfax Laboratories Pty Ltd). For reference, a copy of the laboratory report is attached in Appendix C.

The input data sheets used in the modelling are presented in Appendix D.

9.4.2 Pollutant Attenuation Factors

Natural attenuation of excess effluent nutrient loads from a LAA will occur within the underlying soil and groundwater, providing reductions in contaminant concentrations to mitigate off-site export.

Pollutant attenuation rates for hydraulic, nitrogen and phosphorus loads are adopted from Table 10.7 in the DSC DAF Technical Manual (2015). These attenuation rates have been established through modelling undertaken in several case studies for the inland/rolling hills and coastal/estuarine regions of the LGA and depending on whether DSC prescribed setbacks are achievable.

Based on the location and soil characteristics of the property, the 'inland / rolling hills' catchment scenario has been adopted, with attenuation rates of 40% for hydraulics, 90% for nitrogen, 98% for phosphorus and 99% for pathogens considered appropriate based on achieving 50% of standard setbacks.

All proposed LAAs are outside the 40m buffer to intermittent watercourses and dams, compliant with Note 2 of Table 10.7 in the DSC DAF Technical Manual (2015).

9.4.3 Results and Compliance

Hydraulic and nutrient generation is divided into surplus loads discharged to the ground surface as 'surface surcharge' or draining below the root zone with subsequent (eventual) groundwater migration to surface water bodies or aquifers as 'deep drainage'. The following sections outline the results of the modelling and their compliance with the required acceptance criteria.

The model was run to confirm that the proposed OSSM system can sustainably assimilate the projected wastewater loads.

Copies of all LAM inputs and output results are presented in Appendix D.

9.4.3.1 Hydraulic Loads

Modelling of the movement of water, from both applied effluent (based from the "LAA volume" column of the flow balancing spreadsheet in Appendix B) and rainfall, through the soil is a key component of the LAM. The table below presents the mean annual overflow, surface surcharge and deep drainage predicted for the 60-year modelling period.

Parameter	Run001
Run Description	60 year modelled flow
Total LAA (m ²)	1,200m ²
Surface Surcharge Frequency (days/year)	5.7
Surface Surcharge as % total WWF	2.9
Deep Drainage (mm/day)	6.11

The modelling results show that surface surcharge is not expected to occur for an average WWF day. With approximately 209 days of the year being below average and many other

days being slightly higher the LAA will rarely see runoff (surcharge) frequency. During the proposed high WWF period during the rodeo, agricultural show and Christmas & Easter breaks the LAA can feasibly be dosed at **16,000L** a day (with use of balance tanks). Low effluent dose rates prior to the events will allow for sufficient drying out of the beds and the underlying soil, this will reduce the chance of surcharge.

The modelling shows that the LAA will be in danger of surcharging the most during successive high dose days over the Christmas break with slightly less chance during either the 2-day Local Show or the 3-day Rodeo. The rest of the year (~350 days) shows little to no surcharge chance. Thus, the DSC DAF (2015) requirement of 95% containment via deep drainage and evapotranspiration is achieved.

9.4.3.2 Nutrient and Pathogen Results

The following table summarises the predicted mean annual nutrient and pathogen loads generated by the LAA design and potentially released beyond the LAA footprint.

Parameter	TP (kg/yr)	TN (kg/yr)	Total Virus (MPN/L)
Deep Drainage Output	42.4	5.2	4.5
Surface Surcharge Output	1.0	0.2	N/A

LAM modelling shows that nutrient export through surface surcharge is not expected or unlikely to occur through the OSSM system. Deep drainage is the principal pathway for nutrient export for the design model run.

9.5 Effluent Flow Balancing

As shown in the graph in Appendix B the Site experiences high variability in wastewater load generation, it is common to introduce 'flow balancing' to ensure a more constant daily load of effluent to the Land Application Area, and to manage diurnal (daily) and seasonal fluctuations. This involves the installation of effluent storage tanks to hold excess effluent during busy periods and eliminate surge flows that can cause overloading of the LAA. It also allows for optimisation of LAA by incrementally dosing the LAA of 'peak' flows upon entering lower generation periods.

To determine the required size of effluent storage necessary to adequately balance the expected hydraulic loads from the Site, an 18-month flow balancing analysis was prepared (copy attached at Appendix B). The analysis is used iteratively to determine an optimal balance between effluent storage volume and LAA loading capacity, taking into consideration the variable generation volumes estimated throughout the operating year and EMA available on-site.

Based on EMA analysis in Section 6 W&A determined that $1,600m^2$ of available EMA was present at the Site. A daily water balance was carried out and determined that a safe maximum loading rate onto the available EMA would equate to ~16,000L /day.

One pump-out will occur before the Rodeo (see Section 8.4.1).

Based on abovementioned information; a maximum LAA loading rate of 16,000L, a 20,000L pump-out prior to the Rodeo and predicted wastewater generation over an 18-month period (see section 5), the analysis determined that 71,436L of storage is needed at the Site to ensure that the LAA will not be overloaded.

This is rounded to **75,000L** of actual storage within flow balancing tanks. This ensures that a LAA dose rate of **16,000L/day** can be managed for high-flow periods throughout the year. Effluent storage levels within the balance tanks throughout an 18-month period are shown within the 'cumulative wastewater storage' column as well as graphically in Appendix B.

Effluent storage ensures that the LAA is not overloaded. An overloaded LAA will cause pooling or seepage which will cause damage to the soil structure resulting in the transfer of wastewater contaminants to environmentally sensitive features like the Allyn River.

9.5.1 Balance tanks

W&A propose that 75,000L of effluent storage during large events on-site will be accomplished via 3 (three) 25,000L concrete tanks (see Figure 3, Appendix A). These tanks are proposed to sit on a level pad and will be connected in a way that ensures a storage set-up which reflects the sporadic flow at the Site. An irrigation plan provided in the final approval documentation will specify how this optimization can occur.

9.6 Absorption Bed Construction

The proposed beds must be installed above the existing ground surface to achieve adequate separation from the most limiting soil horizon. The preferred arrangement comprises the construction of 6 pressure-dosed beds having dimensions of 4.0m (width), 0.3m (depth) and lengths ranging from 54-47.5m. With each bed laterally spaced at 1.5m, this arrangement equates to 1,194m² of LAA (1,200m² rounded) of bed "basal" area within the EMA. These will be constructed within a raised platform of good quality moderately to strongly-structured loam (certified to receive effluent) 1 metre high (to achieve a minimum separation of 600mm between the base of the bed and limiting soil horizon) with a 3:1 batter extending down from all sides.

The beds should be constructed in accordance with Appendix L in AS/NZS1547:2012 and the construction diagram presented as Figure 4 in Appendix A of this WMR.

The beds must be installed by a professional experienced in wastewater to ensure that effluent is distributed evenly across the entire area serviced. The finished ground surface of the beds should be slightly mounded to allow for settling to occur. The installer should also be careful to ensure that the minimum buffer distances from the LAA to property boundaries and the road are met.

9.6.1 Fill

W&A have determined that the best approach is to import clean, good-quality (Cat 3) loam fill to re-construct the raised-bed LAA. This will support a DLR (Design Loading rate) of 15 L/m²/day (AS/NZ 1547:2012) and allow for sustainable land application of effluent during 'peak' generation periods (reducing storage requirements).

The raised bed with a 3:1 batter will need to be 1m high to adequately provide ~600mm of free draining soil from the base of the bed to the natural surface. The volume of fill required is estimated as follows:

- Main area: 1,600m² x 1m = 1,600m³
- Batter: 162m [perimeter] x 1m x 3m x 0.5 = 243m³
- Bed volume will be 0.3m x ~1,200m² = 360m³
- Total: 1,600 + 243 360 = 1,483m³

Therefore, the total fill required for bed construction will be 1,500m³ (rounded)

9.6.2 Effluent dosing

A suitable effluent dosing method is via Low Pressure Effluent Dosing (LPED) lines in conjunction with a suitably sized (timer activated) external pump and automatic sequencing valves. This dosing method will provide effective distribution over the basal area of the beds while avoiding potential spot loading associated with perforated gravity distribution lines. Pressure distribution can be achieved by either drilled 25-32mm PVC pipe sleeved with 100mm slotted PVC pipe or 90mm agricultural drainage pipe. Manual flush valves (in valve box) must be fitted to the terminal end of the pressurised distribution manifold in the beds to ensure fouling of the pressure distribution laterals does not occur.

The effluent storage tanks should also be fitted with a high-water level alarm incorporating an audible (buzzer) and visual (strobe light or similar) alarm components alerting Management of an operational problem.

The land application system should be installed by a licenced plumber experienced in wastewater, ensuring that effluent is distributed evenly across the entire area serviced. The absorption beds should be constructed in accordance Figure 4 in Appendix A.

9.7 LAA Positioning

The preferred location of the LAA is identified east of the block 4 amenities. This area will be specified as a Section 88b. instrument as a prescribed effluent disposal area.

9.8 Reserve LAA

Council may require nomination of a reserve LAA in the event of future problems with the preferred land application system installed. The provision of a reserve LAA is NOT achievable within the available EMA (Figure 1, Appendix A).

10 Mitigation Measures

10.1 Soil Improvement

Given that Site soils are identified as moderately dispersive and have low phosphorus sorption capacity they may impact vegetative growth and soil stability within the LAA. These properties can combine to reduce the soils' capacity to sustainably manage wastewater.

Prolonged application of sodium rich wastewater can exacerbate the situation. Application of calcium mineral is a recognised way of reducing the effects of soil instability. It does this by supplying calcium to the affected soil and thereby elevating calcium concentrations with respect to sodium. Added calcium will improve the soil CEC and Ca/Mg ratio, improving fertility, while reducing the potential for soil structural degradation.

Calcium in the form of gypsum is recommended to be applied on the LAA. Gypsum is only slowly soluble in water, so simply broadcasting at the surface can be of limited benefit as it can take a long time for the calcium to penetrate the soil and reach the deeper soil layers. It is therefore recommended to incorporate the amendment into the soil during construction of the land application system. This will be done by scattering the gypsum at the top surface of the natural soil before the loam fill is placed. A suitable gypsum application rate of approximately 0.5kg/m² should be applied.

10.2 Vegetation Establishment

Vegetation that is suited to the application of effluent, preferably with high water and nutrient requirements (such as turf) should be established over the LAAs immediately following

construction. A complete vegetation cover is important to reduce the erosion hazard and optimise water and nutrient uptake. It is recommended to establish and maintain a vegetated buffer around the LAAs. It should be planted with moisture-tolerant vegetation and remain well maintained to maximise moisture uptake. Plants must be selected that will not be so large as to shade the LAAs once fully grown. It is important that the LAAs receive maximum exposure to sun and wind to maximise evapotranspiration.

To maximise assimilation of effluent-borne nutrients within the LAAs, vegetation clippings should be removed from the LAAs and mulched elsewhere on-site for use on other landscaped areas that are not used for wastewater application. Mulching clippings back onto the area from which they were cut is not recommended. An alternative is to dispose clippings in the general waste bin, or green waste bin collection service, if provided.

10.3 Water Saving Measures

To minimise wastewater generation, it is recommended that all domestic water use fixtures in each new dwelling be installed in accordance with BASIX requirements, including installation of 'standard water reduction fittings.

Standard water reduction fixtures for internal and external water use include:

- Taps WELS 4-star (or better) rated;
- Toilets 4.5/3.0 litre dual flush pan and cistern;
- Showers WELS 3-star (or better) rated; and
- Dishwashers (if used) AAA rated using as little as 18 litres per wash.

Implementation of these measures is expected to reduce water use, and thereby wastewater generation, by as much as 10-15%.

11 Conclusions and Recommendations

This completes our assessment of the Site's capability for sustainable OSSM in relation to the W&A audit of the existing system as well as proposed developments at Gresford Showground - 29 Park Street, East Gresford, NSW and presents suitable options for OSSM servicing at the Site. Specifically, W&A recommend the following:

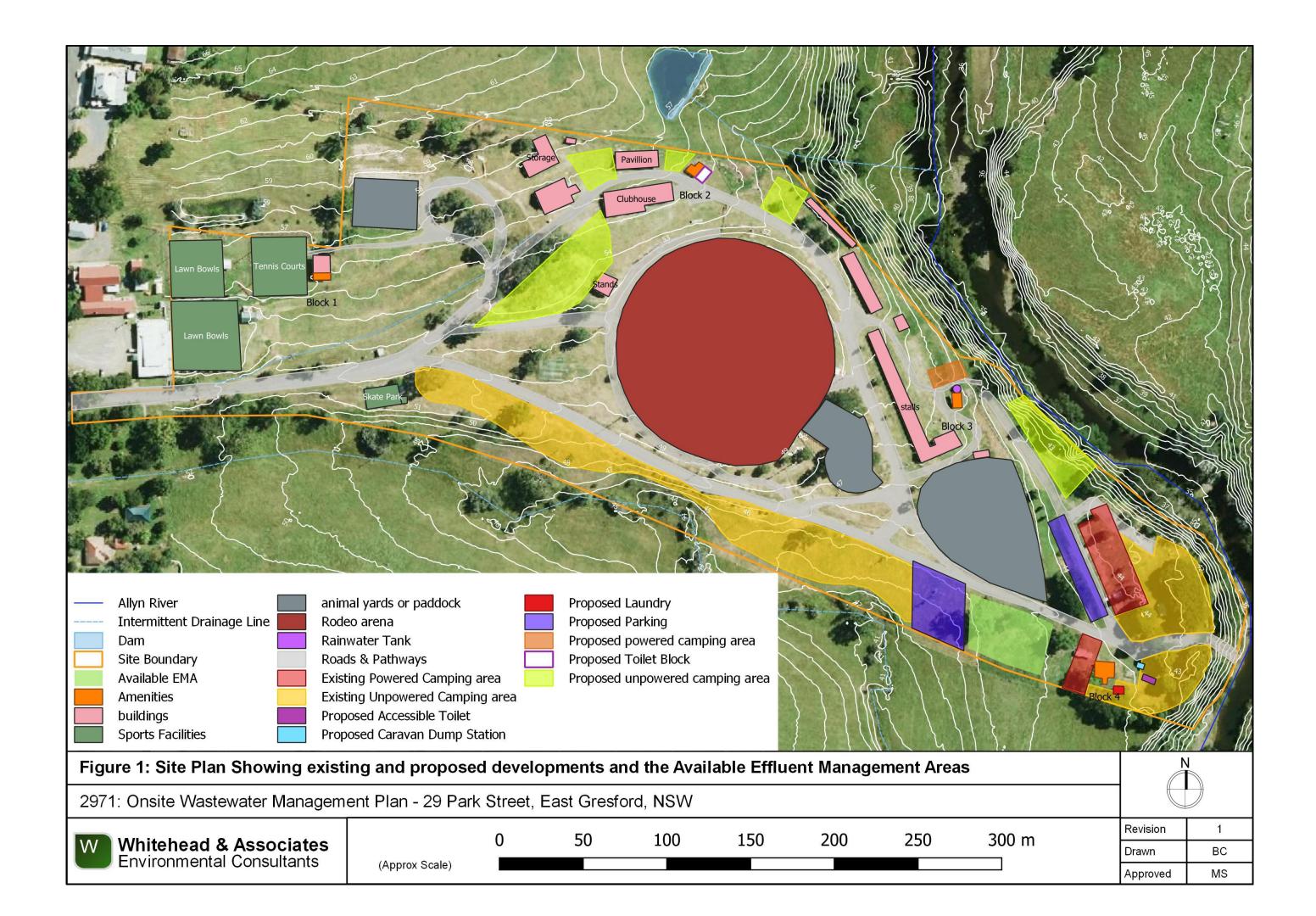
- Wastewater generated at the Site will be treated to a primary standard.
- The OSSM system will be upgraded and reconfigured to consolidate the treatment of wastewater servicing Block 2, 3 & 4. The proposed changes will include:
 - decommissioning of redundant tanks at Blocks 2 and 3;
 - installation of new 10,000L interceptor (septic) tanks at Blocks 2 and 3;
 - installation of a new 10,000L pump well at Block 2;
 - replacement of existing pump-sets installed in pump wells at Blocks 2, 3 and 4 to include dual macerator pump assemblies;
 - installation of 2 (two) new 25,000L septic tanks to receive all effluent generated from the 3 amenities blocks; and
 - installation of 3 (three) new flow balance tanks (totalling 75,000L) to temporarily store treated effluent prior to land application.
- Installation of a (minimum) <u>1,200m²</u> of <u>raised absorption beds</u> is recommended and must be located within the available EMA specified to comply with adopted setbacks from surface waters, property boundaries and other improvements (NSW DLG, 1998);
- Special controls will be installed to evenly distribute effluent over the entire bed LAA with LPED dosed periodically from the effluent balance tanks via an external pump (with timed controls);
- A good quality (loam) topsoil must be imported and installed 1m high across the whole LAA to achieve a minimum separation of 600mm between the base of the bed and limiting soil horizons;
- A suitable gypsum application rate of approximately 0.5kg/m² should be applied at the base of the land application systems during installation;
- Vegetation must be established over the LAA immediately after installation; and
- Vehicles and grazing animals must be prevented from entering the designated LAA.

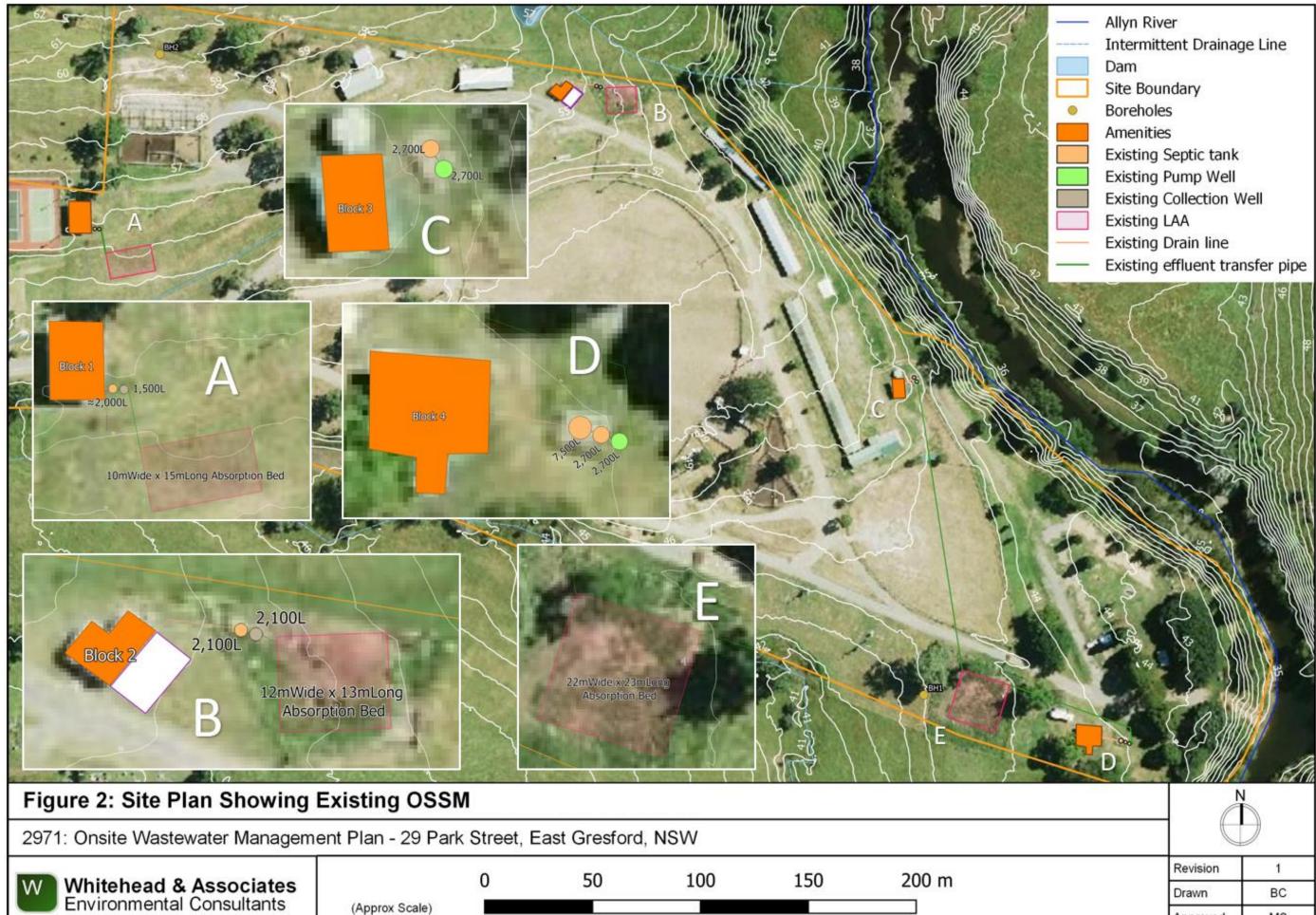
Yours Sincerely,

Ben Colautti Environmental Consultant Whitehead & Associates Environmental Consultants Pty Ltd

Appendix A

Figures





(Approx Scale)

Revision	1	
Drawn	BC	
Approved	MS	

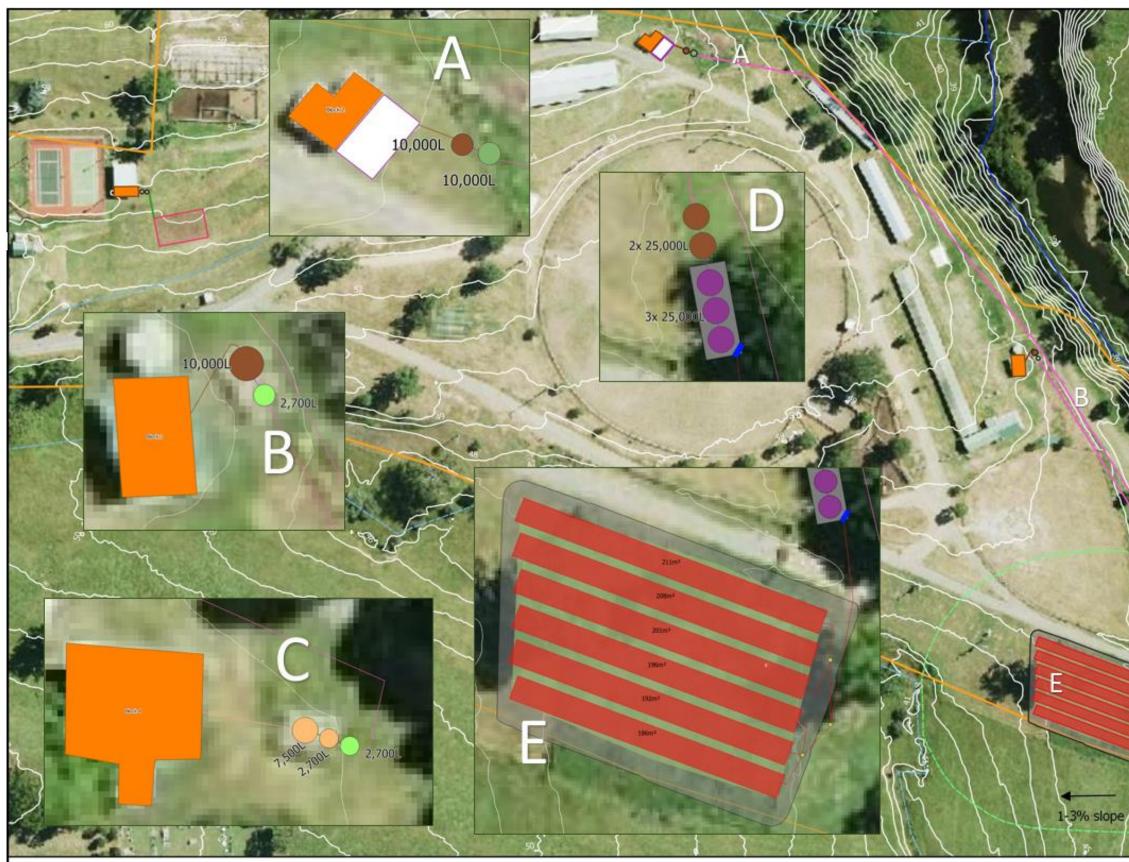


Figure 3: Site Plan Showing Proposed OSSM & EMA

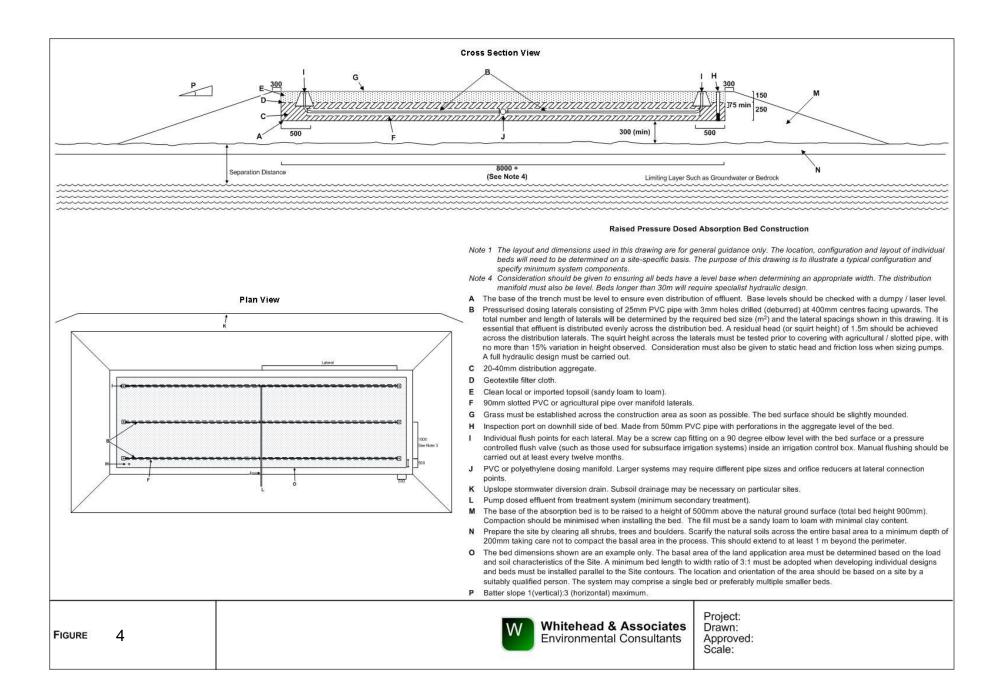
2971: Onsite Wastewater Management Plan - 29 Park Street, East Gresford, NSW

Whitehead & Associates Environmental Consultants (Approx Scale) 0 50 100 150 200 m

Dam
Allyn River
Intermittent Drainage Line
Site Boundary
Available EMA
Amenities
Proposed Toilet Block
Existing Septic tank
Existing Collection Well
Existing Pump Well
Existing LAA
Proposed Septic Tank
Proposed Pump Well
Proposed Balance tank
Index Valve
External Pump
Proposed Absorption Beds LAA
Batter & Fill Area
Level Pad
Nutrient Buffer
Effluent Transfer (Gravity)
Effluent Transfer (Pressure)
Existing Drain line
Existing effluent transfer pipe
Proposed Drain line
Proposed Rising Main
a set production of the set of th



)
Revision	2
Drawn	BC
Approved	MS



Appendix B

Wastewater Generation &

Flow Balancing

OSSM 1 - Block 1

	Source	Typical Wastewater Flow Design Allowance (L/p/day) ¹	Unit	Number	Design Wastewater Flow (L/day)
Tennis Courts	Weekday	10	players	8	80
	Weekend day	10	players	20	200
	Peak day	10	players	100	1,000

Block 2, 3 & 4 (shoulder season)

	Source	Typical Wastewater Flow Design Allowance (L/p/day) ¹	Unit	Number	Design Wastewater Flow (L/day)
Skate Park	Weekday	10	Skaters	10	100
Skale Falk	Weekend day	10	Skaters	30	300
Pit Stop/	Weekday	10	visitor	15	150
recreational	Weekend day	10	visitor	30	300
	Powered Campers (weekday)	75	Camper	62	4,680
Comping	Powered Campers (Weekend day)	75	Camper	124	9,300
Camping	Unpowered Campers (weekday)	27	Camper	0	0
	Unpowered Campers (Weekend day)	27	Camper	32	864
				gn Weekday (L/d) /eekend day (L/d)	

	Source	Typical Wastewater Flow Design Allowance (L/p/day) ¹	Unit	Number	Design Wastewater Flow (L/day)
Skate Park	Weekday	10	Skaters	7	70
Skate Park	Weekend day	10	Skaters	21	210
Pit Stop	Weekday	10	visitor	11	105
	Weekend day	10	visitor	21	210
	Powered Campers (weekday)	75	Camper	10	780
Comping	Powered Campers (Weekend day)	75	Camper	124	9,300
Camping	Unpowered Campers (weekday)	27	Camper	0	0
	Unpowered Campers (Weekend day)	27	Camper	6	162
			Desig	955	
			Design W	9,882	

Block 2, 3 & 4 (low season)

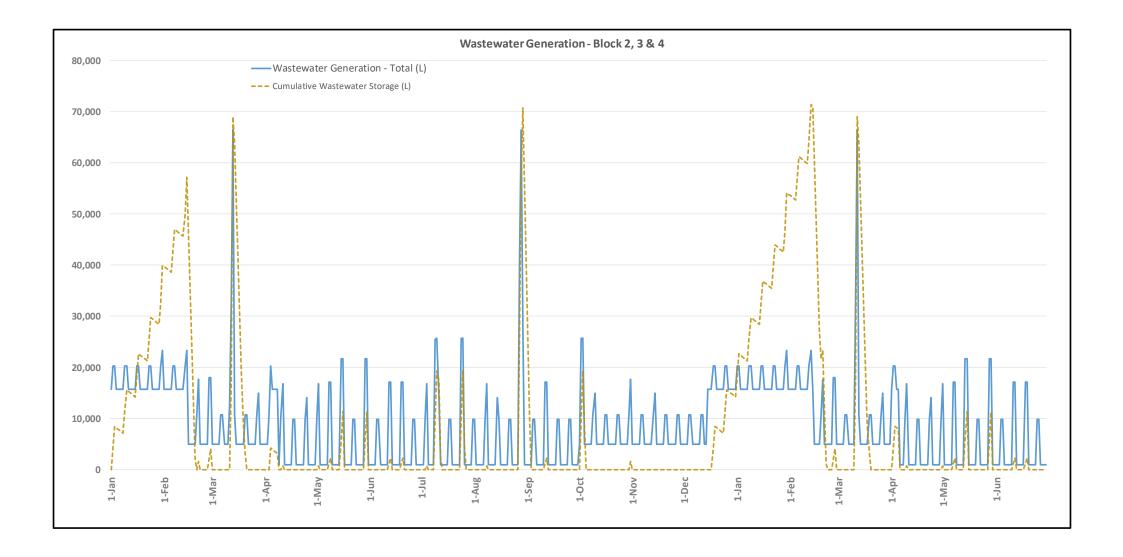
Block 2, 3 & 4 (high season)

	Source	Typical Wastewater Flow Design Allowance (L/p/day) ¹	Unit	Number	Design Wastewater Flow (L/day)
Skate Park	Weekday	10	Skaters	13	130
Skale Park	Weekend day	10	Skaters	39	390
Dit Stop	Weekday	10	visitor	20	195
Pit Stop	Weekend day	10	visitor	39	390
	Powered Campers (weekday)	75	Camper	124	9,300
Camping	Powered Campers (Weekend day)	75	Camper	124	9,300
Camping	Unpowered Campers (weekday)	27	Camper	226	6,102
	Unpowered Campers (Weekend day)	27	Camper	376	10,152
			Design	Weekday (L/d)	15,727
			Design Wee	ekend day (L/d)	20,232

						WAST	EWATER	GENEF	ATION			\rightarrow If there	is effluent in the	balance tank the lo	oad rate is 16000	DL If not, the load	rate is the W	W generatio	n value for that	: day	Total Pumpouts (1 year)		
Date	Day	Season	Event	Event Att	tendee Camping	Non-event Camper	Total Campers on-site	Block 2	Block 3	Block 4	Wastewater Generation - Total (L)	LAA volume (L)	Stored Wastewater (L)	Stored Wastewater from Previous Day (L)	Cumulative Wastewater Storage (L)	Cumulative Storage Managed by Pumpout (L)	Pump out	Peak Day	Maximum Storage Requiremen	(1)	High flow period Shoulder period Low flow period	Average 17,100 7,910 5,410	Peak 23,330 66,600 66,420
1-Jan 2-Jan 3-Jan	friday saturday sunday	high high high		Attendee 0 0 0	Attendee 0 0 0	350 500 500	350 500 500	325 780 780	924 1167 1167	14478 18285 18285	15727 20232 20232	16,000 15,727 16,000 16,000	0 4,232 4,232	0 0 4,232	0 4,232 8,464	0 4,232 8,464	20,000	66,592	71,436	8,656			
4-Jan 5-Jan 6-Jan 7-Jan 8-Jan	monday tuesday wednesday thursday friday	high high high high high		0 0 0 0	0 0 0 0	350 350 350 350 350	350 350 350 350 350	325 325 325 325 325 325	924 924 924 924 924 924	14478 14478 14478 14478 14478	15727 15727 15727 15727 15727	16,000 16,000 16,000 16,000 16,000	-273 -273 -273 -273 -273	8,464 8,191 7,918 7,645 7,372	8,191 7,918 7,645 7,372 7,099	8,191 7,918 7,645 7,372 7,099							
9-Jan 10-Jan 11-Jan 12-Jan	saturday sunday monday tuesday	high high high high high		0 0 0 0	0 0 0	500 500 350 350	350 500 500 350 350	325 780 780 325 325	924 1167 1167 924 924	18285 18285 18285 14478 14478	15727 20232 20232 15727 15727	16,000 16,000 16,000 16,000	-273 4,232 4,232 -273 -273	7,372 7,099 11,331 15,563 15,290	7,099 11,331 15,563 15,290 15,017	7,099 11,331 15,563 15,290 15,017							
13-Jan 14-Jan 15-Jan 16-Jan	wednesday thursday friday saturday	high high high high		0 0 0 0	0 0 0	350 350 350 350	350 350 350 500	325 325 325 780	924 924 924 1167	14478 14478 14478 18285	15727 15727 15727 20232	16,000 16,000 16,000 16,000	-273 -273 -273 4,232	15,017 14,744 14,471 14,198	14,744 14,471 14,198 18,430	14,744 14,471 14,198 18,430							
17-Jan 18-Jan 19-Jan 20-Jan	sunday monday tuesday wednesday	high high high high		0 0 0	0 0 0	500 350 350 350	500 350 350 350	780 325 325 325	1167 924 924 924	18285 14478 14478 14478	20232 15727 15727 15727	16,000 16,000 16,000 16,000	4,232 -273 -273 -273	18,430 22,662 22,389 22,116	22,662 22,389 22,116 21,843	22,662 22,389 22,116 21,843							
21-Jan 22-Jan 23-Jan 24-Jan	thursday friday saturday sunday monday	high high high high		0 0 0 0	0 0 0 0	350 350 500 500	350 350 500 500	325 325 780 780 325	924 924 1167 1167 924	14478 14478 18285 18285 14478	15727 15727 20232 20232	16,000 16,000 16,000 16,000	-273 -273 4,232 4,232	21,843 21,570 21,297 25,529	21,570 21,297 25,529 29,761	21,570 21,297 25,529 29,761							
25-Jan 26-Jan 27-Jan 28-Jan 29-Jan	tuesday wednesday thursday friday	high high high high high		0 0 0 0	0 0 0	350 350 350 350 350	350 350 350 350 350	325 325 325 325 325 325	924 924 924 924 924	14478 14478 14478 14478 14478	15727 15727 15727 15727 15727	16,000 16,000 16,000 16,000 16,000	-273 -273 -273 -273 -273	29,761 29,488 29,215 28,942 28,669	29,488 29,215 28,942 28,669 28,396	29,488 29,215 28,942 28,669 28,396							
30-Jan 31-Jan 1-Feb 2-Feb	saturday sunday monday tuesday	high high high high	Pony Club (PRPC) (1 d)	0 240 0 0	0 60 0 0	500 500 350 350	500 520 350 350	780 2693 325 325	1167 1837 924 924	18285 18792 14478 14478	20232 23322 15727 15727	16,000 16,000 16,000 16,000	4,232 7,322 -273 -273	28,396 32,628 39,950 39,677	32,628 39,950 39,677 39,404	32,628 39,950 39,677 39,404					January		
3-Feb 4-Feb 5-Feb 6-Feb	wednesday thursday friday saturday	high high high high		0 0 0	0 0 0	350 350 350 500	350 350 350 500	325 325 325 780	924 924 924 1167	14478 14478 14478 18285	15727 15727 15727 20232	16,000 16,000 16,000 16,000	-273 -273 -273 4,232	39,404 39,131 38,858 38,585	39,131 38,858 38,585 42,817	39,131 38,858 38,585 42,817							
7-Feb 8-Feb 9-Feb 10-Feb 11-Feb	sunday monday tuesday wednesday thursday	high high high high high		0 0 0 0	0 0 0 0	500 350 350 350 350	500 350 350 350 350	780 325 325 325 325	1167 924 924 924 924 924	18285 14478 14478 14478 14478	20232 15727 15727 15727 15727	16,000 16,000 16,000 16,000 16,000	4,232 -273 -273 -273 -273 -273	42,817 47,049 46,776 46,503 46,230	47,049 46,776 46,503 46,230 45,957	47,049 46,776 46,503 46,230 45,957							
12-Feb 13-Feb 14-Feb 15-Feb	friday saturday sunday monday	high high high Shoulder	Pony Club (PRPC) (1 d)	0 0 240 0	0 0 60 0	350 350 500 500 62	350 350 500 520 62	325 780 2693 250	924 1167 1837 281	14478 18285 18792 4399	15727 15727 20232 23322 4930	16,000 16,000 16,000 16,000	-273 -273 4,232 7,322 -11,070	45,957 45,684 49,916 57,238	45,684 49,916 57,238 46,168	45,684 49,916 57,238 46,168							
16-Feb 17-Feb 18-Feb 19-Feb	tuesday wednesday thursday friday	Shoulder Shoulder Shoulder Shoulder	r r	0 0 0	0 0 0	62 62 62 62	62 62 62 62	250 250 250 250	281 281 281 281	4399 4399 4399 4399	4930 4930 4930 4930	16,000 16,000 16,000 16,000	-11,070 -11,070 -11,070 -11,070	46,168 35,098 24,028 12,958	35,098 24,028 12,958 1,888	35,098 24,028 12,958 1,888							
20-Feb 21-Feb 22-Feb 23-Feb	saturday sunday monday tuesday	Shoulder Shoulder Shoulder Shoulder	r Dressage r r	0 400 0 0	0 100 0 0	156 156 62 62	156 256 62 62	600 3735 250 250	610 1817 281 281	9554 12092 4399 4399	10764 17644 4930 4930	16,000 16,000 16,000 4,930	-5,236 1,644 -11,070 0	1,888 0 1,644 0	0 1,644 0 0	0 1,644 0 0							
24-Feb 25-Feb 26-Feb 27-Feb 28-Feb	wednesday thursday friday saturday sunday	Shoulder Shoulder Shoulder Shoulder Shoulder	r r F PRPC Zone	0 0 60 60	0 0 240 240	62 62 156 156	62 62 396 396	250 250 250 1185 1185	281 281 281 1194 1194	4399 4399 4399 15645 15645	4930 4930 4930 18024 18024	4,930 4,930 4,930 16,000 16,000	0 0 2,024 2,024	0 0 0 2,024	0 0 2,024 4,048	0 0 2,024 4,048					February		
1-Mar 2-Mar 3-Mar 4-Mar	monday tuesday wednesday thursday	Shoulder Shoulder Shoulder Shoulder Shoulder	r r r	0 0 0 0	0 0 0 0	62 62 62 62 62	62 62 62 62 62	250 250 250 250 250	281 281 281 281 281	4399 4399 4399 4399 4399	4930 4930 4930 4930 4930	16,000 16,000 4,930 4,930 4,930	-11,070 0 0 0	4,048 0 0 0	0 0 0 0 0	0 0 0 0 0							-
5-Mar 6-Mar 7-Mar 8-Mar	friday saturday sunday monday	Shoulder Shoulder Shoulder Shoulder	r r	0 0 0	0 0 0	62 156 156 62	62 156 156 62	250 600 600 250	281 610 610 281	4399 9554 9554 4399	4930 10764 10764 4930	4,930 10,764 10,764 4,930	0 0 0 0	0 0 0 0	0 0 0	0 0 0 0							
9-Mar 10-Mar 11-Mar 12-Mar	tuesday wednesday thursday friday	Shoulder Shoulder Shoulder Shoulder	r r f Show (day 1)	0 0 50 1800	0 0 150 200	62 62 62 62	62 62 212 262	250 250 625 14650	281 281 826 5582	4399 4399 10986 12255	4930 4930 12437 32487	4,930 4,930 12,437 15,000	0 0 0 17,487	0 0 0	0 0 0 17,487	0 0 0 17,487							
13-Mar 14-Mar 15-Mar 16-Mar 17-Mar	saturday sunday monday tuesday wednesday	Shoulder Shoulder Shoulder Shoulder Shoulder	r r	4480 0 0 0 0	520 0 0 0 0	156 156 62 62 62	520 156 62 62 62	35100 600 250 250 250	12700 610 281 281 281	18792 9554 4399 4399 4399	66592 10764 4930 4930 4930	15,000 16,000 16,000 16,000 16,000	51,592 -5,236 -11,070 -11,070 -11,070	17,487 69,079 63,843 52,773 41,703	69,079 63,843 52,773 41,703 30,633	69,079 63,843 52,773 41,703 30,633							
18-Mar 19-Mar 20-Mar 21-Mar	thursday friday saturday sunday	Shoulder Shoulder Shoulder Shoulder Shoulder	r r r	0 0 0 0	0 0 0 0	62 62 156 156	62 62 156 156	250 250 250 600 600	281 281 610 610	4399 4399 4399 9554 9554	4930 4930 10764 10764	16,000 16,000 16,000 16,000	-11,070 -11,070 -5,236 -5,236	41,703 30,633 19,563 8,493 3,257	30,833 19,563 8,493 3,257 0	19,563 8,493 3,257 0							
22-Mar 23-Mar 24-Mar 25-Mar	monday tuesday wednesday thursday	Shoulder Shoulder Shoulder Shoulder	r r r	0 0 0 0	0 0 0	62 62 62 62	62 62 62 62	250 250 250 250	281 281 281 281	4399 4399 4399 4399	4930 4930 4930 4930	4,930 4,930 4,930 4,930	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0							
26-Mar 27-Mar 28-Mar 29-Mar	friday saturday sunday monday	Shoulder Shoulder Shoulder Shoulder	r F Pony Club (PRPC) (1 d) r	0 0 240 0	0 0 60 0	62 156 156 62	62 156 216 62	250 600 2513 250	281 610 1345 281	4399 9554 11077 4399	4930 10764 14934 4930	4,930 10,764 14,934 4,930	0 0 0	0 0 0	0 0 0	0 0 0 0							
30-Mar 31-Mar 1-Apr 2-Apr	tuesday wednesday thursday friday saturday	Shoulder Shoulder Shoulder Shoulder Shoulder	r r r	0 0 0 0 0	0 0 0 0	62 62 62 62	62 62 62 62	250 250 250 250 600	281 281 281 281 610	4399 4399 4399 4399 9554	4930 4930 4930 4930 10764	4,930 4,930 4,930 4,930 4,930	0 0 0 0	0 0 0 0	0 0 0	0 0 0 0					March		-
3-Apr 4-Apr 5-Apr 6-Apr 7-Apr	sunday monday tuesday wednesday	High High High High		0 0 0 0 0 0	0 0 0 0 0	156 500 350 350 350	156 500 350 350 350	780 325 325 325	924 924 924	18285 14478 14478 14478	10764 20232 15727 15727 15727	10,764 16,000 16,000 16,000 16,000	0 4,232 -273 -273 -273	0 4,232 3,959 3,686	0 4,232 3,959 3,686 3,413	4,232 3,959 3,686 3,413							
8-Apr 9-Apr 10-Apr 11-Apr	thursday friday saturday sunday	High Low Low	RFS	0 0 0 400	0 0 100	350 10 130 130	350 10 130 230	325 175 420 3555	924 47 568 1775	14478 733 8894 11432	15727 955 9882 16762	16,000 16,000 9,882 16,000	-273 -15,045 0 762	3,413 3,140 0 0	3,140 0 0 762	3,140 0 0 762							
12-Apr 13-Apr 14-Apr 15-Apr	monday tuesday wednesday thursday friday	Low Low Low Low Low		0 0 0 0	0 0 0 0	10 10 10 10 10	10 10 10 10 10	175 175 175 175 175	47 47 47 47 47	733 733 733 733 733 733	955 955 955 955 955	16,000 955 955 955 955	-15,045 0 0 0 0	762 0 0 0	0 0 0 0	0 0 0							
16-Apr 17-Apr 18-Apr 19-Apr 20-Apr	saturday sunday monday tuesday	Low Low Low Low		0 0 0 0	0 0 0	130 130 10 10	130 130 10 10	420 420 175 175	568 568 47 47	8894 8894 733 733	9882 9882 955 955	955 9,882 9,882 955 955	0 0 0 0	0 0 0	0 0 0 0	0 0 0 0							
21-Apr 22-Apr 23-Apr 24-Apr	wednesday thursday friday saturday	Low Low Low		0 0 0	0 0 0	10 10 10 130	10 10 10 130	175 175 175 420	47 47 47 568	733 733 733 8894	955 955 955 9882	955 955 955 9,882	0 0 0 0	0 0 0 0	0 0 0	0 0 0 0							
25-Apr 26-Apr 27-Apr 28-Apr	sunday monday tuesday wednesday	Low Low Low Low	Pony Club (PRPC) (1 d)	240 0 0 0	60 0 0	130 10 10 10	190 10 10 10	2333 175 175 175	1302 47 47 47	10417 733 733 733	14052 955 955 955	14,052 955 955 955	0 0 0	0 0 0	0 0 0	0 0 0							
29-Apr 30-Apr 1-May 2-May 3-May	thursday friday saturday sunday monday	Low Low Low Low Low	Dressage	0 0 400 0	0 0 100 0	10 10 130 130 10	10 10 130 230 10	175 175 420 3555 175	47 47 568 1775 47	733 733 8894 11432 733	955 955 9882 16762 955	955 955 9,882 16,000 16,000	0 0 762 -15,045	0 0 0 762	0 0 762 0	0 0 762 0					April		-
4-May 5-May 6-May 7-May	tuesday wednesday thursday friday	Low Low Low Low		0 0 0 0	0 0 0	10 10 10 10	10 10 10 10	175 175 175 175	47 47 47 47 47	733 733 733 733 733	955 955 955 955	955 955 955 955	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0							
8-May 9-May 10-May 11-May	saturday sunday monday tuesday	Low Low Low Low	Pony Club (PRPC) (2 d) Pony Club (PRPC) (2 d)	60 60 0 0	240 240 0 0	130 130 10 10	370 370 10 10	1005 1005 175 175	1152 1152 47 47	14985 14985 733 733	17142 17142 955 955	16,000 16,000 16,000 955	1,142 1,142 -15,045 0	0 1,142 2,284 0	1,142 2,284 0 0	1,142 2,284 0 0							
12-May 13-May 14-May 15-May	wednesday thursday friday saturday sunday	Low Low Low Low Low	Dungog Motorcycle	0 0 100 100	0 0 400 400	10 10 10 130 130	10 10 10 520 520	175 175 175 1395 1395	47 47 47 1525 1525	733 733 733 18792 18792	955 955 955 21712 21712	955 955 955 16,000	0 0 5,712 5,712	0 0 0 5,712	0 0 5,712 11,424	0 0 5,712							
16-May 17-May 18-May 19-May 20-May	tuesday wednesday thursday	Low Low Low Low	Dungog Motorcycle	0 0 0 0	0 0 0 0	10 10 10 10	10 10 10 10	175 175 175 175	47 47 47 47 47	733 733 733 733 733	955 955 955 955	16,000 16,000 955 955 955	-15,045 0 0	5,712 11,424 0 0	0 0 0 0	11,424 0 0 0 0							
21-May 22-May 23-May 24-May	friday saturday sunday monday	Low Low Low Low		0 0 0 0	0 0 0	10 130 130 10	10 130 130 10	175 420 420 175	47 568 568 47	733 8894 8894 733	955 9882 9882 955	955 9,882 9,882 955	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0							
25-May 26-May 27-May 28-May	tuesday wednesday thursday friday	Low Low Low Low		0 0 0	0 0 0	10 10 10 10	10 10 10 10	175 175 175 175	47 47 47 47	733 733 733 733	955 955 955 955	955 955 955 955	0 0 0 0	0 0 0	0 0 0	0 0 0							
29-May 30-May 31-May 1-Jun 2- Jun	saturday sunday monday tuesday wednesday	Low Low Low Low Low	Penning Penning	100 100 0 0 0	400 400 0 0 0	130 130 10 10	520 520 10 10	1350 1350 175 175 175	1510 1510 47 47 47	18792 18792 733 733 733	21652 21652 955 955 955	16,000 16,000 16,000 955 955	5,652 5,652 -15,045 0	0 5,652 11,304 0	5,652 11,304 0 0	5,652 11,304 0 0					Мау		
2-Jun 3-Jun 4-Jun 5-Jun 6-Jun	wednesday thursday friday saturday sunday	Low Low Low Low Low		0 0 0 0	0 0 0 0	10 10 130 130	10 10 10 130 130	175 175 175 420 420	47 47 47 568 568	733 733 733 8894 8894	955 955 955 9882 9882	955 955 9,882 9,882	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0							
7-Jun 8-Jun 9-Jun 10-Jun	monday tuesday wednesday thursday	Low Low Low		0 0 0	0 0 0	10 10 10 10	10 10 10 10	175 175 175 175	47 47 47 47	733 733 733 733	955 955 955 955	955 955 955 955	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0							
11-Jun 12-Jun 13-Jun 14-Jun	friday saturday sunday monday	Low Low Low	Pony Club (PRPC) (2 d) Pony Club (PRPC) (2 d)	0 60 60 0	0 240 240 0	10 130 130 10	10 370 370 10	175 1005 1005 175	47 1152 1152 47	733 14985 14985 733	955 17142 17142 955	955 16,000 16,000 16,000	0 1,142 1,142 -15,045	0 0 1,142 2,284	0 1,142 2,284 0	0 1,142 2,284 0							
15-Jun 16-Jun 17-Jun 18-Jun	tuesday wednesday thursday friday saturday	Low Low Low Low Low		0 0 0 60	0 0 0 240	10 10 10 10	10 10 10 10 370	175 175 175 175 1005	47 47 47 47 1152	733 733 733 733 14985	955 955 955 955 17142	955 955 955 955	0 0 0 0 1 142	0 0 0	0 0 0 1 142	0 0 0 1 142							
19-Jun 20-Jun 21-Jun 22-Jun 23-Jun	saturday sunday monday tuesday wednesday	Low Low Low Low Low	PRPC Zone PRPC Zone	60 60 0 0	240 240 0 0 0	130 130 10 10 10	370 370 10 10 10	1005 1005 175 175 175	1152 1152 47 47 47 47	14985 14985 733 733 733 733	17142 17142 955 955 955	16,000 16,000 16,000 955 955	1,142 1,142 -15,045 0 0	0 1,142 2,284 0 0	1,142 2,284 0 0 0	1,142 2,284 0 0 0							
24-Jun 25-Jun 26-Jun 27-Jun	thursday friday saturday sunday	Low Low Low Low		0 0 0 0	0 0 0	10 10 130 130	10 10 130 130	175 175 420 420	47 47 568 568	733 733 8894 8894	955 955 9882 9882	955 955 9,882 9,882	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0							
28-Jun 29-Jun 30-Jun	monday tuesday wednesday	Low Low Low		0 0 0	0 0 0	10 10 10	10 10 10	175 175 175	47 47 47	733 733 733	955 955 955	955 955 955	0 0 0	0 0 0	0 0 0	0 0 0					June		

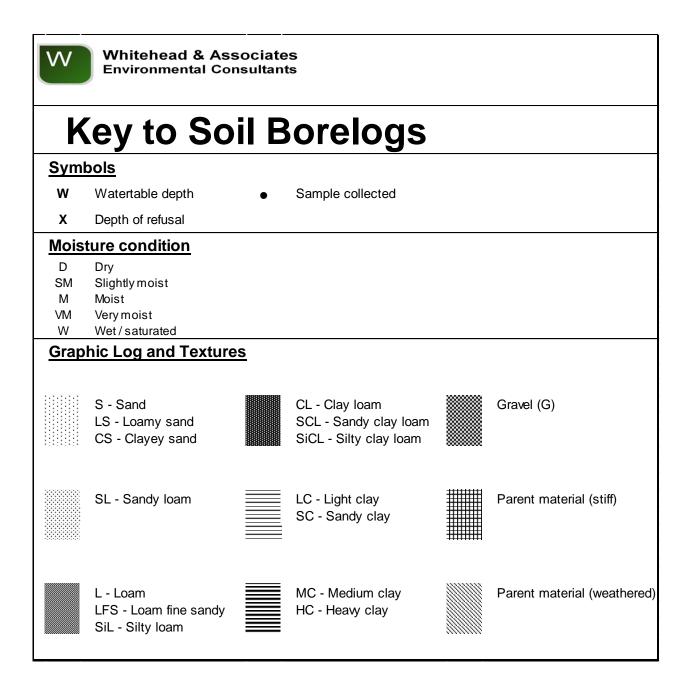
1-Jul 2-Jul 3-Jul 4-Jul 5-Jul	friday Lu saturday Lu sunday Lu	DW DW DW Dressage DW	0 0 0 0 400 10 0 0	0 10 0 130 00 130 0 10	10 1 130 4 230 3 10 1	75 47 733 75 47 733 20 568 8894 555 1775 11432 75 47 733	955 955 9882 16762 955	955 955 9,882 16,000 16,000	0 0 762 -15,045	0 0 0 762	0 0 762 0	0 0 762 0		
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14-Jul V	wednesday Lu thursday Lu friday Lu saturday Lu sunday Lu monday Lu	20W 20W 20W 20W 20W 20W 20W		10 10 10 10 10 10 130 130 10	10 1 10 1 130 4 130 4 130 1	175 47 733 75 47 733 75 47 733 75 47 733 20 568 8894 20 568 8894 75 47 733 75 47 733 75 47 733	955 955 955 9882 9882 955 955	955 955 955 9,882 9,882 9,882 955 955	-15,045 0 0 0 0 0 0 0	2,266 0 0 0 0 0 0 0 0				
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	wednesday Lu thursday Lu friday Lu saturday Lu sunday Lu monday Lu	DW DW DW DW DW Dressage DW DW	0 0 0 0 0 0 400 10 0 0 0 0	10 10 0 10 0 10 0 130 00 130 00 130 0 10	10 1 10 1 10 1 130 4 230 3 10 1	75 47 733 75 47 733 75 47 733 20 568 8894 555 1775 11432 75 47 733 75 47 733	955 955 955 9882	955 955 955 9,882 16,000 16,000 955	0 0 0 762 -15,045 0	0 0 0 0 762 0	0 0 0 762 0 0	0 0 0 762 0 0		
12-Aug 13-Aug 14-Aug 15-Aug 16-Aug 17-Aug	thursday Li friday Li saturday Li sunday Li monday Li	DW DW DW Pony Club (PRPC) (1 d) DW DW DW	0 0 0 0 240 6 0 0 0 0 0 0	10 10 10 10 130 130 130 130 130	10 1 10 1 190 2 130 4 10 1 10 1	75 47 733 75 47 733 75 47 733 333 1302 10417 20 568 8894 75 47 733 75 47 733	955 955 955 14052 9882 955 955	955 955 955 14,052 9,882 955 955		0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0		
19-Aug 20-Aug 21-Aug 22-Aug 23-Aug 24-Aug	thursday Lu friday Lu saturday Lu sunday Lu monday Lu tuesday Lu	20W 20M 20M 20M 20M 20M 20M		10 10 10 10 130 130 130 10 10 10 10 10 10 10 10 10 10 10 10	10 1 10 1 130 4 130 4 10 1 10 1	75 47 733 75 47 733 75 47 733 75 47 733 20 568 8894 20 568 8894 75 47 733 75 47 733 75 47 733	955 955 9882 9882 9882 955 955	955 955 9,882 9,882 9,882 955 955	0 0 0 0 0 0 0		0 0 0 0 0 0 0	0 0 0 0 0 0		
26-Aug 27-Aug 28-Aug 29-Aug 30-Aug 31-Aug	thursday Lu friday Lu saturday Lu sunday Lu monday Lu tuesday Lu	DW Rodeo (day 1) DW Rodeo (day 1) DW Rodeo (day 2) DW Rodeo (day 3) DW DW	0 0 0 0 2800 20 4480 52 1950 5 0 0 0 0 0 0	0 10 00 10 20 130 0 130 0 10 0 10 0 10	10 1 200 22 520 34 100 15 10 1 10 1	75 47 733 75 47 733 075 7981 10671 920 12700 18792 945 5625 7050 75 47 733 75 47 733 75 47 733		955 955 15,000 15,000 16,000 16,000 16,000	0 0 25,727 51,412 13,620 -15,045 -15,045	0 0 5,727 57,139 70,759 55,714 40,669	0 0 25,727 57,139 70,759 55,714 40,669 25,624	0 0 5,727 57,139 70,759 55,714 40,669 25,624	1	August
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Appendix C

Soil Borelogs and Data Summary



SOIL BORE LOG



Whitehead & Associates Environmental Consultants

(Client:	Emma Elo	dridge	of Gresford	d Park Trust		Borehole	No:	BH1	
	Site:	Gresford Sh	now gro	und; 29 Park	Street, East Gres	ford, NSW	Excavated/lo	ogged by:	Connor Mor	ton & Ben Colautti
	Date:	28th July					Excavation t	ype:	Auger, crow	/bar & shovel
Ν	lotes:	- refer to s	site pla	an for positi	on of borehole					
					PROFIL	E DESCR	RIPTION			
Depth (m)	Graphic Log	Sampling depth/name	Horizon	Texture	Structure	Colour	Mottles/ Gley	Coarse Fragments	Moisture Condition	Comments
0.1		BH1/1	A1	LC	weak	very dark	N/A	20-50%	М	Large Cobble in
0.1						grayish brown		(<200mm)		Profile
0.2										
0.3										
0.5										
0.4		BH1/2	B1	MC	massive	dark brown		10-20%	М	cobble in profile
0.5							orange/light brown	(<60mm)		
0.6										
0.7										
0.8										
0.9		Refusal or	n Float	ter						
1										
1.1										
1.2										

C	Client:	Emma Ele	dridge	of Gresford	Park Trust		Borehole		BH2				
	Site:			ound; 29 Park	Street, East Gres	ford, NSW	Excavated/I		ł	ton & Ben Colautti			
	Date: lotes:	28th July		n for pociti	on of borehole		Excavation type: Auger, crowbar & shovel						
					PROFIL	E DESC		1					
Depth (m)	Graphic Log	Sampling depth/name	Horizon	Texture	Structure	Colour	Mottles/ Gley	Coarse Fragments	Moisture Condition	Comments			
0.1		BH2/1	A	SC	massive	brown	N/A	N/A	D	dry compacted san Thin layer of massiv clay before bedroc			
0.3 0.4 0.5 0.6 0.7		Refusal or	n Pare	nt material	(Stiff)								
0.8 0.9 1													

0	Client:				Park Trust		Borehole		BH3	
	Site:		v	ound; 29 Park	Street, East Gres	ford, NSW	Excavated/I			on & Ben Colaut
	Date: lotes:	28th July - refer to s		an for positi	on of borehole		Excavation	type:	Auger, crowb	ar & snovel
		r			PROFIL	E DESCI		1	, , , , , , , , , , , , , , , , , , , ,	
Depth (m)	Graphic Log	Sampling depth/name	Horizon	Texture	Structure	Colour	Mottles/ Gley	Coarse Fragments	Moisture Condition	Comments
0.1		BH3/1	A	SCL	weak	dark yellowish brown	N/A	20-50%	D	compacted
0.3 0.4 0.5		Refusal or	n Pare	ent material	(Weathered)					
0.6										
0.8 0.9										

	Soil Sampling Schedule and Results of pH, EC and Emerson Aggregate Test Analysis														
Site	Name (mm) Class [4] [4] [4] [4] [4] [4] [4] [4] [4] [4]														
BH1	BH1/1	350	LC	3(2)	Low	n/a	6.73	Neutral	98	0.78	Non-saline	Ca, Mg, Na, K, P-Sorb, CEC & ESP			
	BH1/2	850	MC	5	Low	n/a	6.78	Neutral	47	0.33	Non-saline				
	Ref		F							0.00					
BH2	BH2/1	300	SC	2(1)	Mod	n/a	6.85	Neutral	75	0.00	Non-saline				
	Ref		PM(S)							0.00					
BH3	BH3/1	350	SCL	2(1)	Mod	n/a	6.84	Neutral	83	0.00	Non-saline				
	Ref		PM(W)							0.00					

Notes: - (also refer Interpretation Sheet 1)

[1] The modified Emerson Aggregate Test (EAT) provides an indication of soil susceptibility to dispersion.

[2] Ratings describe the likely hazard associated with land application of treated wastewater.

[3] pH measured in the field using Raupac Indicator.

[4] pH measured on 1:5 soil:water suspensions using a Hanna Combo hand-held pH/EC/temp meter.

[5] Electrical conductivity of the saturated extract (Ece) = $EC_{1:5}(\mu S/cm) \times MF / 1000$. Units are dS/m. MF is a soil texture multiplication factor.

[6] External laboratories used for the following analyses, if indicated:

CEC (Cation exchange capacity)

• Psorb (Phosphorus sorption capacity)

Bray Phosphorus

Organic carbon

Total nitrogen

Sheet 2 - Results of External Laborator	y Analysis
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Site	Name	Depth (mm)	CEC (me/100g)	Rating	Ca (mg/kg)	Rating	Mg (mg/kg)	Rating	Na (mg/kg)	Rating	K (mg/kg)	Rating	ESP (%)	Rating	P-sorp. (mg/kg)	Rating
1471_East Gresford_2015	1471	400	9.1	L	1214	М	300	М	36	L	45	VL	1.7	NS	111	Н

Phone Office/Lab (02) 6775 1157

email: <u>lanfaxlabs@bigpond.com.au</u> Website: <u>http://www.lanfaxlabs.com.au</u> Lab address: 493 Old Inverell Road Postal address: PO Box 4690 Armidale NSW 2350 Director: Dr Robert Patterson FIEAust, CPSS, CPAg Soil Scientists and Environmental Engineers



5th June 2015

Whitehead & Associates 197 Main Road Cardiff NSW 2285

Soil Report: Job No. 1471 Sample received 1st June 2015, sample date Samples dried to 50°C, crushed and sieved to minus 2 mm prior to analysis

Whitehead & Assoc. Job 1471 27MAY15

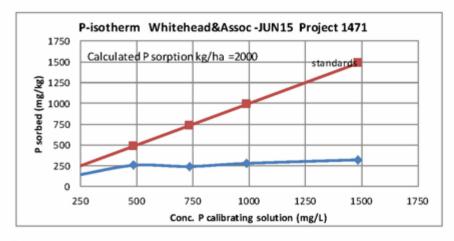
Exc.Al+ H		Ca		к	h	Ng		Na	Base Sat.	ESP	P CEC Ca/		Site Location
cmol+/kg	mg/kg	cmol+/kg	mg/kg	cmol+/kg	mg/kg	cmol+/kg	mg/kg	cmol+/kg	56	5%	omol+/kg	ratio	Sample ID
0.3	1214	6.06	45	0.12	300	2.47	36	0.15	96.5	1.7	9.1	2.4	Whitehead & Assoc. 1471

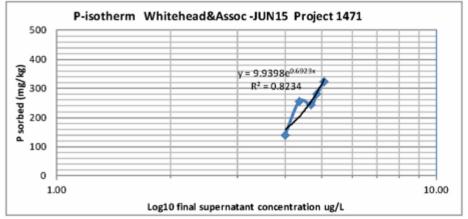
Methods: Rayment & Lyons 2011 P sorption modified method 9J1 - elevated equilibrating solutions, ICP determination of P Cations: Method 15D3, no pretreatment Exchangeable Acidity: Method 15G1

Yours faithfully,

atterson

Dr Robert Patterson FIEAust, CPSS(3), CPAg Soil Scientist and Environmental Engineer





Percent sorbed	is the proport	tion of the ini	tial P sorbed during equilibration	on	P-isother	m Whitehe	ad&Assoc -	JUN15 Pro
Initial P	filtrate	sorbed P	Sample	Percent	Std line	filtrate	Yaxis	X axis
mgP/L	Р	mg/kg	I.D.	sorbed		С	Log C	
	mg/L			(%)		ugP/L		
24.0	10.00	139.5	Whitehead&Assoc -JUN15	58.3	240	9999	4.00	139.5
48.7	23.09	256.2	Project 1471	52.6	487	23090	4.36	256.2
73.4	49.13	242.7		33.1	734	49133	4.69	242.7
99.0	71.00	279.8		28.3	990	70997	4.85	279.8
148.4	116.14	322.6		21.7	1484	116143	5.06	322.6
Calcul	ated P sorpti	on kg/ha =	2000					

Appendix D

Water and Nutrient Balance Modelling

Emt wbm Land	Application Mana	gement To	ool				Run Model
Site Data	Soil Data			Layer # (S	Single La	yer Versio	on)
Application Area (m²)1200Land Application Type3Storage Type1Application Method1Storage Capacity (m³)0Storage Depth (m)0Average Slope (%)1Soil Typepa_fillCrop TypeDefault	Effective Satur Field Capacity Permanent Wi Saturated Hyd Soil Depth for Bulk Density (I Depression Str Infiltration Rat Infiltration Exp Coefficient P So Exponent P De	(mm) Iting Point (r raulic Condu P Sorption (r gg/m ³) orage (mm) e (mm/day) ponent orption rption	uctivity (mm/day) m)	1 414.0 300.0 1500.0 0.6 1400.0 0.0 200.0 1.5 79.3 0.30 0.15			
Land Application and Acceptance Rates	Crop Data	Add New	Meteorological Dat	a		,	/iew Data
Storage Seepage (mm/day) 0 Fixed Application Depth (mm) 0	January February	1	Number of Years	60.8 <i>R</i>	ET	E	Т
Soil Water Trigger (mm) 0	March	1	Max	221.6	9.3	16.5	35.2
Additional Application Depth (mm) 0	April	1	Min	0.0	0.4	0.2	6.1
Nitrogen Crop Uptake (kg/ha/yr) 260 Phosphorus Crop Uptake (kg/ha/yr) 30	May June	1	Average Median	2.5 0.0	3.4 3.1	4.0 3.6	18.0 18.0
Wastewater Characteristics	July	1	Standard Deviation	8.1	1.7	2.2	4.9
Constant Daily WWF (m³/day) 0 Total Nitrogen (mg/L) 60 Use WWF Total Phosphorus (mg/L) 15 timeseries Virus (MPN/L) 100 instead?	Yes August September Voctober November 3/day December	1 1 1 1 1	ONLY grey cell Refer to comme	s require i	nput.]	

BMT WBM

Land Application Management Tool

View Timeseries Results

Summary of Results

Runoff (surcharge) frequency Runoff (surcharge) volume Deep drainage volume	5.7 days/year2.9 % of total WWF volume2675.4 m3/yr6.11 mm/day
Total phosphorus load in runoff	1.0 kg/yr
Total nitrogen load in runoff	0.2 kg/yr
Total phosphorus load in deep drainage	42.5 kg/yr
PO4 concentration in deep drainage	11.5 g/cub.m
Total nitrogen load in deep drainage	5.2 kg/yr
NO3 concentration in deep drainage	1.3 g/cub.m
Total site virus load 12	423721 MPN/yr
Total site virus concentration	4.5 MPN/L
Total site phosphorus load	43.5 kg/yr
Total site nitrogen load	5.4 kg/yr
Storage overflow frequency Storage overflow volume	 0 number of years 0.0 days/year 0.0 cub.m/yr 0.0 % of total WWF volume

Appendix E

General Notes

2021 Cal	endar				Calendarpedia Your source for calendars
January	February	March	April	May	June
1 Fr New Year's Day	1 Mo	1 Mo	1 Th	1 Sa	1 Tu
2 Sa	2 Tu	2 Tu	2 Fr	2 Su DRESSAGE	2 We
3 Su	3 We	3 We	3 Sa	3 Mo	3 Th
4 Mo	4 Th	4 Th	4 Su	4 Tu	4 Fr
5 Tu	5 Fr	5 Fr	5 Mo	5 We	5 Sa
6 We	6 Sa	6 Sa	6 Tu	6 Th	6 Su
7 Th	7 Su	7 Su	7 We	7 Fr	7 Mo
8 Fr	8 Mo	8 Mo	8 Th	8 Sa PRPC	8 Tu
9 Sa	9 Tu	9 Tu	9 Fr	9 Su PRPC	9 We
10 Su	10 We	10 We	10 Sa	10 Mo	10 Th
11 Mo	11 Th	11 Th	11 Su Gresford RFS Exe	11 Tu	11 Fr
12 Tu	12 Fr	12 Fr GDAS SHOW	12 Mo	12 We	12 Sa PRPC
13 We	13 Sa	13 Sa GDAS SHOW	13 Tu	13 Th	13 Su PRPC
14 Th	14 Su PRPC	14 Su	14 We	14 Fr	14 Mo
15 Fr	15 Mo	15 Mo	15 Th	15 Sa Dungog Motorcycle	15 Tu
16 Sa	16 Tu	16 Tu	16 Fr	16 Su Dungog Motorcycle	16 We
17 Su	17 We	17 We	17 Sa	17 Mo	17 Th
18 Mo	18 Th	18 Th	18 Su	18 Tu	18 Fr
19 Tu	19 Fr	19 Fr	19 Mo	19 We	19 Sa PRPC Zone Day
20 We	20 Sa	20 Sa	20 Tu	20 Th	20 Su PRPC Zone Day
21 Th	21 Su DRESSAGE	21 Su	21 We	21 Fr	21 Mo
22 Fr	22 Mo	22 Mo	22 Th	22 Sa	22 Tu
23 Sa	23 Tu	23 Tu	23 Fr	23 Su	23 We
24 Su	24 We	24 We	24 Sa	24 Mo	24 Th
25 Mo	25 Th	25 Th	25 Su PRPC	25 Tu	25 Fr
26 Tu	26 Fr	26 Fr	26 Mo	26 We	26 Sa Function booked
27 We	27 Sa PRPC ZONE	27 Sa	27 Tu	27 Th	27 Su
28 Th	28 Su PRPC ZONE	28 Su PRPC	28 We	28 Fr	28 Mo
29 Fr		29 Mo	29 Th	29 Sa PENNING tbc	29 Tu
30 Sa		30 Tu	30 Fr	30 Su PENNING the	30 We
31 Su PRPC		31 We		31 Mo	

2021 Calendar

Calendarpedia Your source for calendars

July	August	September	October	November	December		
1 Th	1 Su	1 We	1 Fr	1 Mo	1 We		
2 Fr	2 Mo	2 Th	2 Sa E' ZONE TBC	2 Tu	2 Th		
3 Sa	3 Tu	3 Fr	3 Su E' ZONE TBC	3 We	3 Fr		
4 Su DRESSAGE	4 We	4 Sa	4 Mo	4 Th	4 Sa		
5 Mo	5 Th	5 Su	5 Tu	5 Fr	5 Su		
6 Tu	6 Fr	6 Mo	6 We	6 Sa	6 Mo		
7 We	7 Sa	7 Tu	7 Th	7 Su	7 Tu		
8 Th	8 Su DRESSAGE	8 We	8 Fr	8 Mo	8 We		
9 Fr American Motor Cyc	9 Mo	9 Th	9 Sa	9 Tu	9 Th		
10 Sa American Motor Cyc	10 Tu	10 Fr	10 Su PRPC	10 We	10 Fr		
11 Su PRPC	11 We	11 Sa PRPC	11 Mo	11 Th	11 Sa		
12 Mo	12 Th	12 Su PRPC	12 Tu	12 Fr	12 Su		
13 Tu	13 Fr	13 Mo	13 We	13 Sa	13 Mo		
14 We	14 Sa PRPC	14 Tu	14 Th	14 Su PRPC	14 Tu		
15 Th	15 Su	15 We	15 Fr	15 Mo	15 We		
16 Fr	16 Mo	16 Th	16 Sa	16 Tu	16 Th		
17 Sa	17 Tu	17 Fr	17 Su	17 We	17 Fr		
18 Su	18 We	18 Sa	18 Mo	18 Th	18 Sa		
19 Mo	19 Th	19 Su	19 Tu	19 Fr	19 Su		
20 Tu	20 Fr	20 Mo	20 We	20 Sa	20 Mo		
21 We	21 Sa	21 Tu	21 Th	21 Su	21 Tu		
22 Th	22 Su	22 We	22 Fr	22 Mo	22 We		
23 Fr	23 Mo	23 Th	23 Sa	23 Tu	23 Th		
24 Sa Mud Run TBC	24 Tu	24 Fr	24 Su	24 We	24 Fr		
25 Su Mud Run TBC	25 We	25 Sa	25 Mo	25 Th	25 Sa Christmas Day		
26 Mo	26 Th	26 Su	26 Tu	26 Fr	26 Su		
27 Tu	27 Fr RODEO	27 Mo	27 We	27 Sa	27 Mo		
28 We	28 Sa RODEO	28 Tu	28 Th	28 Su	28 Tu		
29 Th	29 Su RODEO	29 We	29 Fr	29 Mo	29 We		
30 Fr	30 Mo	30 Th	30 Sa	30 Tu	30 Th		
31 Sa	31 Tu		31 Su DRESSAGE		31 Fr New Year's D. (obs		

Soil Physical Properties / Chemistry

рH

This test is used to determine the acidity or alkalinity of native soils. pH is measured on a scale of 0 to 14, with 7 being neutral. Results below 7 are considered acid, while those above 7 are alkaline. For land application of effluent, soil with a pH of 4.5 to 8.5 should typically pose no constraints. Soil pH affects the solubility and fixation of some nutrients; this in turn reduces soil fertility and plant growth. By correcting soil pH beneficial plant growth is improved, assisting in the assimilation of nutrient and improving evapotranspiration of effluent. Most Australian soils are naturally acidic.

Electrical Conductivity

Electrical conductivity (EC) is a measure of a soil or soil/water extracts ability to conduct an electrical current. It is used as an indirect measure of a soil's accumulation of water-soluble salts, mainly of sodium, with minor potassium, calcium and magnesium. High EC within a land application area reflects general soil salinity and is undesirable for vegetation growth. The tolerance of vegetation species to soil salinity varies among plant types. Typically, EC readings of <4dS/m pose no constraints. There are a number of measures available to counter high soil EC values for land application of effluent; however, the most important measure relates to the conservative selection of application rates and appropriate application area sizing.

Emerson Aggregate Test

The Emerson Aggregate Test (EAT) is a measure of soil dispersibility and susceptibility to erosion and structural degradation. It assesses the physical changes that occur in a single ped of soil when immersed in water, specifically whether the soil slakes and falls apart or disperses and clouds the water. Dispersive soils pose limitations to on-site sewage management because of the potential loss of soil structure when effluent is applied. Soil pores can become smaller or completely blocked, causing a decrease in soil permeability, which can lead to system failure.

Cation Exchange Capacity

The cation exchange capacity (CEC) is the capacity of the soil to hold and exchange cations (positively charged molecules). Because some soils have a dominant negative charge, they can adsorb cations. Soils bind cations such as calcium, magnesium, potassium and sodium, preventing them from being leached from the soil profile and making them available as plant nutrients. CEC is a major controlling agent for soil structural stability, nutrient availability for plants and the soils' reaction to fertilisers and other ameliorants. A CEC of greater than 15 cmol+/kg or me/100g is recommended for land application systems. Adding organic matter (compost/humus) to soil can greatly increase its CEC.

Exchangeable Sodium Percentage

The exchangeable sodium percentage (ESP) is an important indicator of soil sodicity, which affects soil structural stability and overall susceptibility to dispersion. Sodic soils tend to have a low infiltration capability, low hydraulic conductivity, and a high susceptibility to erosion. When sodium dominates the exchangeable cation complex, soil structural stability declines significantly. Soil ESP is considered acceptable for effluent application areas when it is below 5%, marginal between 5% – 10% and limiting >10%. The ESP of application area soils can be improved by the measured application of calcium (lime/gypsum).

Phosphorus Sorption Capacity

Phosphorus sorption (P-sorption) capacity is a direct measure of a soils ability to adsorb phosphorus. Phosphorus is an important plant nutrient and is the limiting available nutrient in many aquatic environments. Excess phosphorus can increase the production of nuisance vegetative growth such as algae. The P-sorption capacity of the soil in an effluent application area relates to its ability to assimilate the phosphorus in the wastewater for the design life of the application area. P-sorption values greater than 400mg/kg is considered acceptable for land application of effluent, while values below 150mg/kg present a constraint.